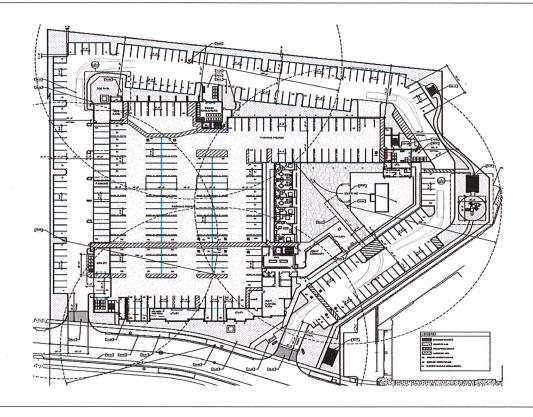
# Appendix J Revised Traffic and Circulation Study

## LOCKWOOD III APARTMENTS PROJECT OXNARD, CALIFORNIA

#### **REVISED TRAFFIC AND CIRCULATION STUDY**



**September 19, 2023** 

**ATE Project 22068** 

#### **Prepared for:**

SVM – Lockwood, LCC 356 Eric Place Thousand Oaks, California 91362

#### Prepared by:

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September 19, 2023

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## REVISED TRAFFIC AND CIRCULATION STUDY FOR THE LOCKWOOD III APARTMENT PROJECT - CITY OF OXNARD

Associated Transportation Engineers (ATE) has prepared the following revised traffic and circulation study for the Lockwood III Apartments Project. The revised traffic and circulation study addresses comments provided by ESA on the October 11, 2022 traffic and circulation study prepared by ATE. It our understanding that the City of Oxnard will use the results of the revised traffic and circulation study to process the Project's development application.

We appreciate the opportunity to assist SVM - Lockwood, LLC with this Project.

**Associated Transportation Engineers** 

By:

Richard L. Pool, P.E.

President

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#### INTRODUCTION

The following report presents the results of the traffic and circulation study completed by Associated Transportation Engineers (ATE) for the Lockwood III Apartments Project, located in the City of Oxnard. The City of Oxnard's traffic study standards were utilized in formatting the various sections of the traffic study. The study provides information relative to Existing, Existing + Project, Cumulative and Cumulative + Project traffic conditions. Site access, onsite parking, constriction traffic and Vehicle Miles Traveled (VMT) are also addressed in the study.

#### PROJECT DESCRIPTION

The Project site is an undeveloped parcel located at 2001 Lockwood Street east of Rose Avenue in the City of Oxnard, as illustrated on Figure 1. The Project is proposing to construct 234 apartment units including 33 affordable units with related on-site amenities. The Project is applying for Density Bonus and related concessions. The Project would require a General Plan Amendment from Business and Research Park (BRP) to High-Rise Residential (R4). Access to the



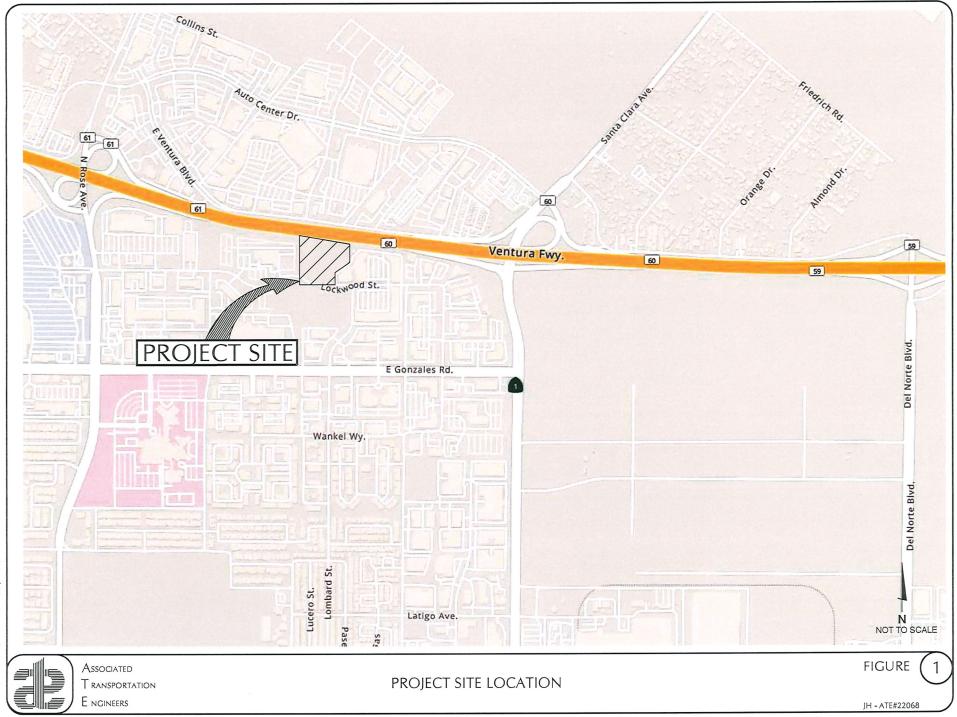
Project site will be provided via two driveway connections to Lockwood Street. The Lockwood III Apartments Project is proposing to provide 351 on-site parking spaces (including 7 ADA parking spaces, 36 electric vehicle charging stations, 5 ADA electric vehicle charging stations and 1 ADA van electric vehicle charging station). The Project site design will provide on-site bicycle storage and bike racks. The Project site plan is illustrated on Figure 2.

#### **EXISTING CONDITIONS**

#### **Existing Street Network**

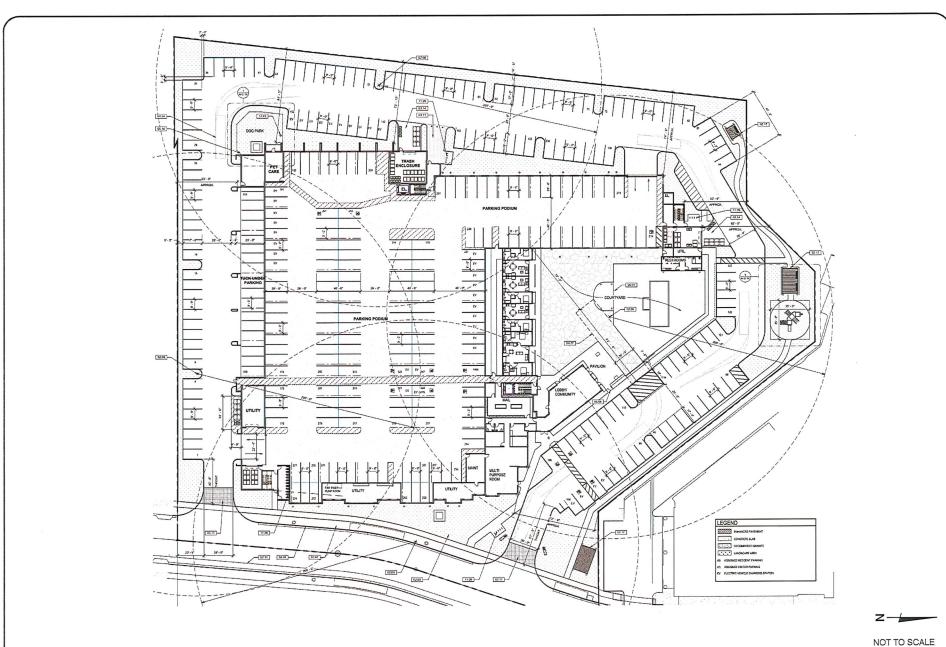
The Project site is served by a circulation system comprised of freeway, arterial and collector streets, which are illustrated on Figure 1 and discussed in the following text.

*U.S. Highway 101*, located north of the Project site, is a multi-lane interstate highway serving the Pacific Coast between the City of Los Angeles and the State of Washington. U.S. Highway 101 is the principal route between the City of Oxnard and the adjacent cities of Ventura to the north and Camarillo to the south. Regional access to the site is provided via the U.S. Highway 101/Rose Avenue and U.S. Highway 101/Rice Avenue interchanges.



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FIGURE



**Rose Avenue** is a 2- to 6-lane north-south roadway that extends north from Sanford Street to State Route 118 (Los Angeles Avenue). Rose Avenue provides a major link between the residential areas in Oxnard and the commercial centers along the U.S. Highway 101 corridor.

*Rice Avenue-Santa Clara Avenue* located east of the Project site is a 2- to 6-lane north-south arterial roadway from State Route 1 to State Route 118 (Los Angeles Avenue). Within the study-area, Rice Avenue-Santa Clara Avenue is signalized at the U.S. Highway 101 ramps, and Gonzales Road intersections. Rice Avenue is designated as a truck route.





Gonzales Road is a 4- to 6-lane east-west divided arterial roadway that serves the north central portions of the City of Oxnard. Gonzales Road extends from Harbor Boulevard to Rice Avenue. Gonzales Road serves both residential, commercial, and medical land-uses in the studyarea. Gonzales Road is designated as a truck route.

Lockwood Street is a 2-lane roadway that extends easterly from Rose Avenue to Solar Drive south of Gonzales Road. Lockwood Street serves retail commercial and office land uses. Lockwood Street links the Project to Shopping at the Rose and Rose Crossing to the west and the Palms Market Place to the east. Direct access to the Project would be provided via two driveway connections on Lockwood Street. Lockwood Street is signalized at Rose Avenue.



**Williams Drive** is a 2-lane roadway that extends southerly from Lockwood Street to Cesar Chavez Drive south of Gonzales Road. Williams Drive serves residential, commercial, and medical office land uses. Williams Drive is signalized at Gonzales Road.



**Solar Drive** is a 2-lane roadway that extends southerly from Lockwood Street to Wankel Way south of Gonzales Road. Solar Drive serves commercial and medical office land uses. Solar Drive is signalized at Gonzales Road.

Lombard Street-Outlet Center Drive is a 2- to 4-lane divided roadway that extends southerly from Lockwood Street to Jacinto Drive. Lombard Street-Outlet Center Drive serves residential, commercial, and medical office uses. Lombard Street-Outlet Center Drive is signalized at Gonzales Road.

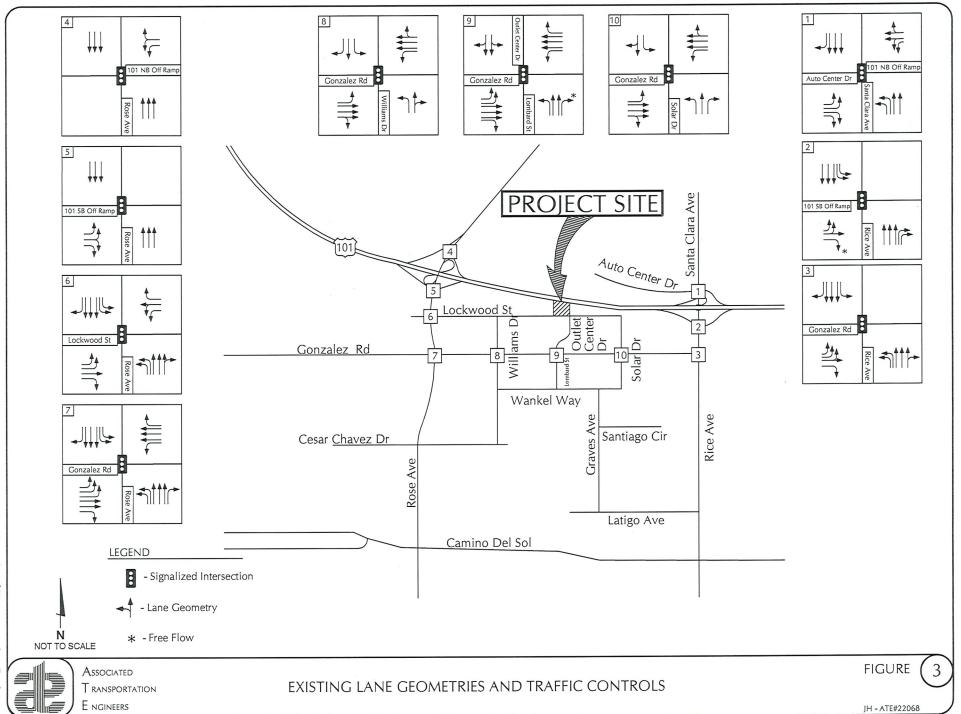


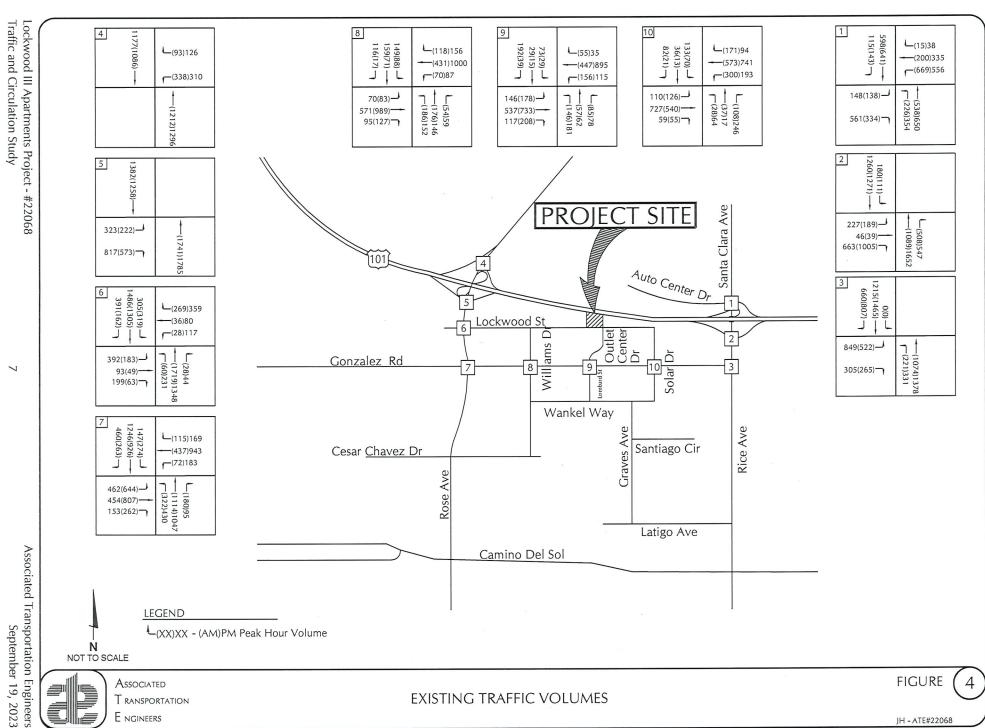
#### **Existing Volumes and Levels of Service**

#### **Intersection Operations**

Traffic flow on urban arterials is most constrained at intersections. Therefore, a detailed analysis of traffic flows must examine the operating conditions of critical intersections during peak travel periods. In rating intersection operations, "Levels of Service" (LOS) "A" through "F" are used, with LOS "A" indicating free flow operations and LOS "F" indicating congested operations (more complete definitions of levels of service are included in the Technical Appendix). In the City of Oxnard LOS "C" is the acceptable operating standard for intersections.

Existing AM and PM peak hour turning movement volumes for the study-area intersections were collected by ATE in September of 2022 (count sheets are contained in Technical Appendix). Figure 3 illustrates the existing traffic controls and geometries for the study-area intersections. The existing Year 2022 AM and PM peak hour traffic volumes at the study-area intersections are illustrated on Figure 4.





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Existing levels of service for the study-area intersections were calculated using the Intersection Capacity Utilization (ICU) methodology for signalized intersections as required by the City of Oxnard. Worksheets illustrating the level of service calculations are contained in the Technical Appendix for reference. Table 1 lists the existing levels of service for the study-area intersections during the AM and PM peak hour periods.

Table 1
Existing Peak Hour Levels of Service

			AM Peak Hour		PM Peak Hour	
Intersection		Control Type	ICU	LOS	ICU	LOS
1.	U.S. Highway 101 NB Ramps/Rice Ave.	Signal	0.49	LOS A	0.61	LOS B
2.	U.S. Highway 101 SB Ramps/Rice Ave.	Signal	0.52	LOS A	0.55	LOS A
3.	Rice Ave./Gonzales Rd.	Signal	0.50	LOS A	0.54	LOS A
4.	U.S. Highway 101 NB Ramps/Rose Ave.	Signal	0.34	LOS A	0.61	LOS B
5.	U.S. Highway 101 SB Ramps/Rose Ave.	Signal	0.53	LOS A	0.61	LOS B
6.	Rose Ave./Lockwood Street	Signal	0.56	LOS A	0.63	LOS B
7.	Rose Ave./Gonzales Rd.	Signal	0.62	LOS B	0.69	LOS B
9.	Gonzales Rd./Williams Dr.	Signal	0.48	LOS A	0.51	LOS A
9.	Gonzales Rd./Lombard St.	Signal	0.37	LOS A	0.44	LOS A
10.	Gonzales Rd./Solar Dr.	Signal	0.42	LOS A	0.48	LOS A

The data presented in Table 1 indicates that the study-area intersections currently operate at LOS B or better during the AM peak hour and PM peak hour periods, which meets the City's LOS C standard.

#### CITY GENERAL PLAN POLICY

<u>City of Oxnard</u>. The City of Oxnard has established LOS C as the acceptable standard for intersection operation. If the addition of project traffic increases the ICU by 0.02 or more at an intersection operating at LOS C or worse, it should be mitigated to the ICU level identified without the project traffic. These criteria were used to determine the effects of the traffic generated by the Project added to the study-area intersections.

#### PROJECT GENERATED TRAFFIC VOLUMES

#### **Project Trip Generation**

Trip generation estimates were calculated for the proposed Lockwood III Apartments based on the rates presented in the Institute of Transportation Engineers (ITE), <u>Trip Generation</u>, 11<sup>th</sup> Edition for Multifamily Housing - Midrise (Land-Use Code #221) and Affordable Housing - Income Limits (Land Use Code #223)<sup>1</sup>. Table 2 summarizes the average daily trips (ADT), AM and PM peak hour trip generation estimates for the proposed housing development.

Table 2 Project Trip Generation

		A	DT	Peak Hour Trips				
				A	AM Peak Hour	PM Pea	ak Hour	
Land Use	#Units	Rate	Trips	Rate Trips		Rate	Trips	
Apartments	201 units	4.54	913	0.38	77 (18 In/59 Out)	0.39	79 (48 In/31 Out)	
Apartments	33 units	7.94	262	0.73	24 (7 In/17 Out)	0.73	24 (14 In/10 Out)	
Total Trip Ge	eneration:		1,175		101 (25 In/76 Out)		103 (62 In/41 Out	

As shown in Table 2 the Project would generate 1,175 average daily trips, 101 AM peak hour trips and 103 PM peak hour trips.

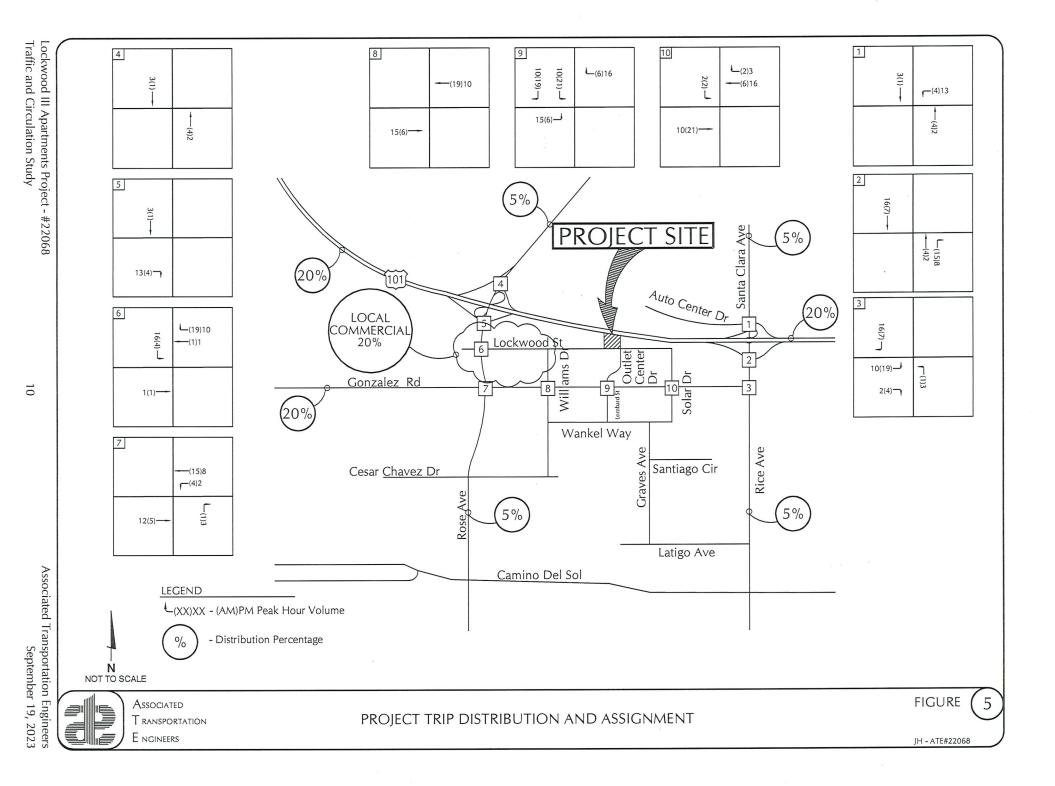
#### Project Trip Distribution and Assignment

The project-generated AM and PM peak hour traffic volumes were distributed and assigned to the study-area intersections based on the existing traffic pattern and general knowledge of the residential, employment, commercial and medical health development in the vicinity of the study-area. Figure 5 illustrates the trip distribution and assignment assumed for the Project's trips, and Figure 6 illustrates the Existing + Project traffic volumes.

#### PROJECT-SPECIFIC ANALYSIS

Levels of service were calculated for the study-area intersections assuming the Existing + Project volumes. Tables 3 and 4 show the results of the calculations and identify the Project's traffic effects based on the City of Oxnard policies.

<sup>&</sup>lt;sup>1</sup><u>Trip Generation</u>, Institute of Transportation Engineers, 11<sup>th</sup> Edition, 2020.



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Table 3
Existing + Project AM Peak Hour Levels of Service

1		Existing		Existing -		
Intersection		ICU	LOS	ICU -	LOS	Change
1.	U.S. Highway 101 NB Ramps/Rice Ave,	0.49	LOS A	0.49	LOS A	0.00
2.	U.S. Highway 101 SB Ramps/Rice Ave.	0.52	LOS A	0.52	LOS A	0.00
3.	Rice Ave./Gonzales Rd.	0.50	LOS A	0.50	LOS A	0.00
4.	4. U.S. Highway 101 NB Ramps/Rose Ave.		LOS A	0.34	LOS A	0.00
5.	U.S. Highway 101 SB Ramps/Rose Ave.	0.53	LOS A	0.53	LOS A	0.00
6.	Rose Ave./Lockwood St.	0.56	LOS A	0.56	LOS A	0.00
7.	Rose Ave./Gonzales Rd.	0.62	LOS B	0.62	LOS B	0.00
8.	8. Gonzales Rd./Williams Dr.		LOS A	0.48	LOS A	0.00
9.	9. Gonzales Rd./Lombard St.		LOS A	0.38	LOS A	0.01
10.	Gonzales Rd./Solar Dr.	0.42	LOS A	0.43	LOS A	0.01

Table 4
Existing + Project PM Peak Hour Levels of Service

		Existing		Existing		
	Intersection	ICU	LOS	ICU	LOS	Change
1.	U.S. Highway 101 NB Ramps/Rice Ave,	0.61	LOS A	0.61	LOS C	0.00
2.	U.S. Highway 101 SB Ramps/Rice Ave.	0.55	LOS A	0.56	LOS B	0.01
3.	Rice Ave./Gonzales Rd.	0.54	LOS A	0.54	LOS B	0.00
4.	4. U.S. Highway 101 NB Ramps/Rose Ave.		LOS B	0.61	LOS A	0.00
5.	U.S. Highway 101 SB Ramps/Rose Ave.	0.61	LOS B	0.61	LOS C	0.00
6.	Rose Ave./Lockwood St.	0.63	LOS B	0.64	LOS C	0.01
7.	Rose Ave./Gonzales Rd.	0.69	LOS B	0.69	LOS C	0.01
8.	Gonzales Rd./Williams Dr.	0.51	LOS A	0.51	LOS A	0.00
9.	Gonzales Rd./Lombard St.	0.44	LOS A	0.45	LOS A	0.01
10.	Gonzales Rd./Solar Dr.	0.48	LOS A	0.49	LOS A	0.01

The data presented in Tables 3 and 4 indicate that with the addition of project-generated traffic the study-area intersections would continue to operate at LOS C or better. The Project would not have an adverse effect on the study-area intersections based on the City of Oxnard's General Plan policy.

#### CUMULATIVE (EXISTING + APPROVED/PENDING PROJECTS) CONDITIONS

The City of Oxnard requires that intersection operations be analyzed with the addition of traffic generated by projects which have been approved or are pending within the Project study-area. Trip generation estimates were developed for the cumulative developments using rates published in the ITE, <u>Trip Generation</u>, 11<sup>th</sup> Edition. Table 5 summarizes the average daily trips, AM and PM peak hour trip generation estimates for the cumulative projects.

Table 5
Approved/Pending Development Projects Trip Generation

					Peak	Hour
No.	Project	Land Use	Size	ADT	AM	PM
1.	631 Douglas Ave.	Apartment	1 Unit	7	1	1
2.	302 Doris Ave.	Apartment	1 Unit	7	1	1
3.	Rio Urbana	Trip Generation fr	om ATE TIA	1,232	107	121
4.	F Street Condos	Condominiums	40 Units	155	9	9
5.	Habitat for Humanity	Single Family Res.	6 Units	57	4	6
6.	Cheyenne Development	Signal Family Res. 3 Units		29	2	3
7.	Jose Corona	Single Family Res.	1 Unit	10	1	1
8.	Oakmont Senior Living	Senior Residential	85 Units	314	17	26
9.	The Village (PA 4)	Condominiums	88 Units	644	40	49
10.	The Village (PA 5 and	Condominiums	78 Units	571	36	44
11.	The Village (PA	Condominiums	144 Units	1,054	66	81
12.	Eddie Alvarado	Single Family Res.	2 Units	19	2	2
13.	Eddie Alvarado	Single Family Res.	2 Units	19	2	2
14.	The Village (PA 18 and	Apartments	219 Units	1,603	101	123
15.	Dioji	Kennel	4,781 S.F.	100	10	10
16.	River Park Hotels	Hotel	240 Rooms	2,006	113	144
1 <i>7</i> .	U-Haul	Vehicle Storage	N/A	500	30	30
18.	Campus at Topa Towers	Trip Generation fr	om ATE TIA	1,737	112	123
19.	Batelaan	Warehouse	3,000 S.F.	11	1	1
20.	Glovis New Car Transit	New Car Storage	3.9 Acres	267	50	76
21.	Audi of Oxnard	Trip Generation fr	om ATE TIA	934	76	97
22.	ALDI	Grocery Store	1,648 S.F.	168	. 6	17
23.	Shoe City	Shopping Center	17,513 S.F.	661	16	67
24.	Cooper Mixed-Use	Apartments	2 Units	15	1	1
25.	Esplanade Gateway	Trip Generation fr	om ATE TIA	762	97	37
26.	5 <sup>th</sup> Street Banquet Hall	Banquet Hall	2,274 S.F.	500	20	50
27.	Gold Coast Transit	Trip Generation from Pe	nfield & Smith TIA	2,263	153	78
28.	Trinity Church	Church	7,400 S.F.	51	2	4
29.	Food 4-Less	Commercial Retail	118,425 S.F.	4,470	111	451
30.	Dewey Pest Control	Office	5,700 S.F.	63	9	8
31.	Third Tower	Office	300,000 S.F.	2,922	348	345
32.	MWS Wire Industries	Warehouse	60,367 S.F.	215	18	19
33.	Cabot Industrial	Warehouse	24,518 S.F.	87	7	8
34.	Pacific Water	Warehouse	25,158 S.F.	90	8	8
35.	Elevar Industrial	Church	36,480 S.F.	332	20	20
36.	Sakioka Farms Specific	Trip Generation from		65,216	7,762	7,528
37.	Mission Produce Office	Trip Generation fr		419	38	32
38.	Santiago at Graves	Trip Generation fr		372	52	47
39.	Lockwood I	Trip Generation fr		640	35	45
39.	Lockwood III	Apartments	154 Units	1,127	71	87
			Total Trips:	91,658	9,555	9,802

The data presented in Table 5 indicate that the approved/pending projects would generate a total of 91,658 average daily trips, 9,555 AM peak hour trips and 9,802 PM peak hour trips. The traffic generated by the approved/pending projects was distributed and assigned to the study-area intersections. The trip assignment for the cumulative development projects was developed based on the location of each project, data presented in recent traffic studies, as

well as a general knowledge of the population, employment and commercial centers in Oxnard and surrounding Ventura County area.

#### **Cumulative Conditions**

Levels of service were calculated for the study-area intersections assuming the Cumulative volumes illustrated on Figure 7. Table 6 lists the Cumulative levels of service for the study-area intersections during the AM and PM peak hour periods. Planned improvements identified in the Sakioka Farms Business Park EIR<sup>1</sup> are assumed to be in place for the Cumulative intersection analysis.

Table 6
Cumulative Peak Hour Levels of Service

			AM Peak Hour		PM Peak Hour	
Intersection		Control Type	ICU	LOS	ICU	LOS
1.	U.S. Highway 101 NB Ramps/Rice Ave.	Signal	0.61	LOS B	0.73	LOS C
2.	U.S. Highway 101 SB Ramps/Rice Ave.	Signal	0.70	LOS B	0.81	LOS D
3.	Rice Ave./Gonzales Rd.	Signal	0.75	LOS C	0.76	LOS C
4.	U.S. Highway 101 NB Ramps/Rose Ave.	Signal	0.40	LOS A	0.57	LOS A
5.	U.S. Highway 101 SB Ramps/Rose Ave.	Signal	0.59	LOS A	0.68	LOS B
6.	Rose Ave./Lockwood St.	Signal	0.61	LOS B	0.71	LOS C
7.	Rose Ave./Gonzales Rd.	Signal	0.81	LOS D	0.78	LOS C
8.	Gonzales Rd./Williams Dr.	Signal	0.75	LOS C	0.75	LOS C
9.	Gonzales Rd./Lombard St.	Signal	0.65	LOS B	0.69	LOS B
10.	Gonzales Rd./Solar Dr.	Signal	0.70	LOS B	0.72	LOS C

The data presented in Table 6 indicate that most of the study-area intersections would operate at LOS C or better during the AM peak hour and PM peak hour periods under Cumulative conditions, which meets the City's LOS C standard. However, the U.S. Highway 101 southbound ramps/Rice Avenue and Rose Avenue/Gonzales Road intersections will operate at LOS D.

<sup>&</sup>lt;sup>1</sup> Sakioka Farms Specific Plan Environmental Impact Report Traffic Study



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#### **Cumulative + Project Impacts**

Levels of service were calculated for the study-area intersections assuming the Cumulative + Project volumes illustrated on Figure 8. Tables 7 and 8 show the results of the calculations and identify the consistency of the Project with the City of Oxnard policies.

Table 7
Cumulative + Project AM Peak Hour Levels of Service

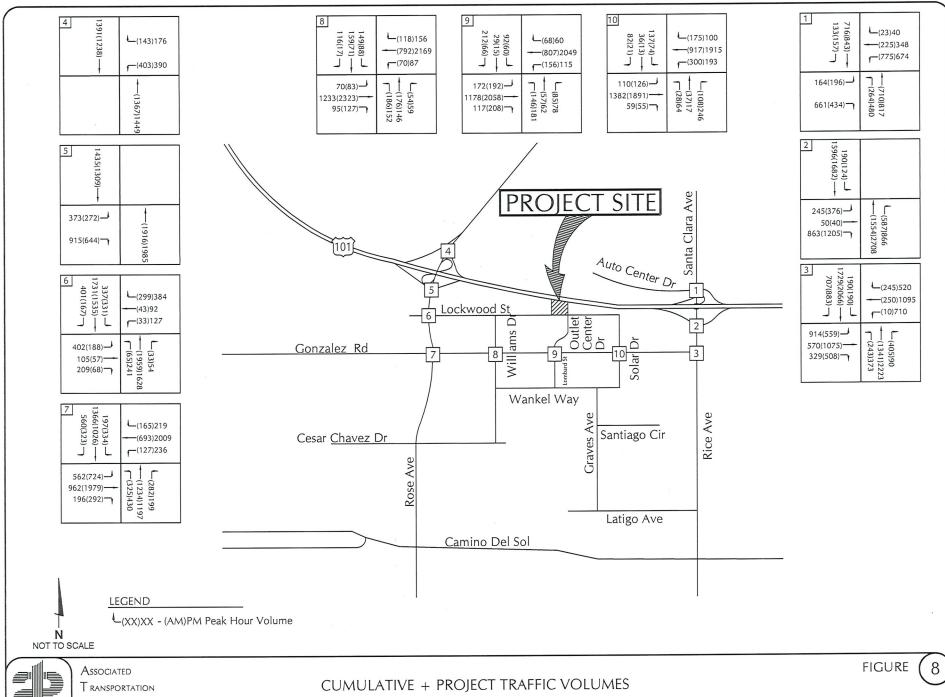
			nulative	Cumulativ		
	Intersection	ICU	LOS	ICU	LOS	Change
1.	U.S. Highway 101 NB Ramps/Rice Ave,	0.61	LOS B	0.61	LOS B	0.00
2.	U.S. Highway 101 SB Ramps/Rice Ave.	0.70	LOS B	0.70	LOS B	0.00
3.	Rice Ave./Gonzales Rd.	0.75	LOS C	0.75	LOS C	0.00
4.	U.S. Highway 101 NB Ramps/Rose Ave.	0.40	LOS A	0.40	LOS A	0.00
5.	U.S. Highway 101 SB Ramps/Rose Ave.	0.59	LOS A	0.59	LOS A	0.00
6.	Rose Ave./Lockwood St.	0.61	LOS B	0.62	LOS B	0.01
7.	Rose Ave./Gonzales Rd.	0.81	LOS D	0.81	LOS D	0.00
8.	Gonzales Rd./Williams Dr.	0.75	LOS C	0.75	LOS C	0.00
9.	Gonzales Rd./Lombard St.	0.65	LOS B	0.66	LOS B	0.01
10.	Gonzales Rd./Solar Dr.	0.70	LOS B	0.71	LOS C	0.01

Table 8
Cumulative + Project PM Peak Hour Levels of Service

			nulative	Cumulative		
	Intersection	ICU	LOS	ICU.	LOS	Change
1.	U.S. Highway 101 NB Ramps/Rice Ave,	0.73	LOS C	0.73	LOS C	0.00
2.	U.S. Highway 101 SB Ramps/Rice Ave.	0.81	LOS D	0.81	LOS D	0.00
3.	Rice Ave./Gonzales Rd.	0.76	LOS C	0.76	LOS C	0.00
4.	U.S. Highway 101 NB Ramps/Rose Ave.	0.57	LOS A	0.57	LOS A	0.00
5.	U.S. Highway 101 SB Ramps/Rose Ave.	0.68	LOS B	0.68	LOS B	0.00
6.	Rose Ave./Lockwood St.	0.71	LOS C	0.71	LOS C	0.00
7.	Rose Ave./Gonzales Rd.	0.78	LOS C	0.78	LOS C	0.00
8.	Gonzales Rd./Williams Dr.	0.75	LOS C	0.75	LOS C	0.00
9.	Gonzales Rd./Lombard St.	0.69	LOS B	0.70	LOS B	0.01
10.	Gonzales Rd./Solar Dr.	0.72	LOS C	0.73	LOS C	0.01

The data presented in Tables 7 and 8 indicate that the project would be consistent with the City of Oxnard's policies during the AM or the PM peak hour periods. Generally, all of the study-area intersections would continue to operate at LOS C or better with the addition of project traffic. The U.S. Highway 101 southbound ramps/Rice Avenue and Rose Avenue/Gonzales Road intersections will continue to operate at LOS D. The Project would not have an adverse effect on the study-area intersections based on the City of Oxnard's General Plan policy (ICU increase of 0.02 or greater).

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CUMULATIVE + PROJECT TRAFFIC VOLUMES

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#### **GENERAL PLAN BUILDOUT**

The Project requires a General Plan Amendment from Business Research Park (BRP) to High Rise Residential (R4). Trip generation estimates were calculated for the proposed land use and the approved land use based on the rates presented in the Institute of Transportation Engineers (ITE), <u>Trip Generation</u>, 11<sup>th</sup> Edition for Multifamily Housing – Mid Rise (Land-Use Code #221), Affordable Housing - Income Limits (Land Use Code #223) and Factory Outlet Center (Land-Use Code #823)<sup>2</sup>. Factory Outlet Center is consistent with the initial traffic study prepared for the entitlement of the existing Palms Market Place. Table 9 summarizes the average daily, AM and PM peak hour trip generation estimates for the proposed and approved General Plan land uses.

Table 9
General Plan Land Use Trip Generation Comparison

		A	DT	,	Weekday Pe	ak Hour Tri	ps
					ak Hour	PM Peak Hour	
Land Use	Size	Rate	Trips	Rate	Trips	Rate	Trips
Apartments	201 units	4.54	913	0.38	77	0.39	79
Apartments	33 units	7.94	<u> 262</u>	0.73	<u>24</u>	0.73	<u>24</u>
		-	1,175		101		103
Factory Outlet Center	120,000 S.F.	26.59	3,191	0.67	80	2.29	275
,	Net	Change:	-2,016		+21		-172

As shown in Table 9 the Project would generate 1,175 average daily trips, 101 AM peak hour trips and 103 PM peak hour trips. The approved land use would generate 3,191 average daily trips, 80 AM peak hour trips and 275 PM peak hour trips. The proposed Lockwood IIII Apartments Project would generate fewer trips average daily and PM peak hour trips than the approved land use. The General Plan Buildout circulation system would be able to accommodate the traffic generated by the Project.

#### SITE ACCESS AND CIRCULATION



As illustrated on Figure 2 (Project Site Plan), access to Lockwood III Apartments Project will be provided by two driveway connections to Lockwood Street. The driveway connections will allow full access to the Project site. Given the existing and projected traffic volumes, the Lockwood Street/Project Driveway intersections will operate acceptably with Project traffic.

<sup>&</sup>lt;sup>2</sup>Trip Generation, Institute of Transportation Engineers, 11<sup>th</sup> Edition, 2020.

The Project driveways would be designed and constructed to City of Oxnard design standards. Lockwood Street is level and straight, adequate sight distance should be provided at the driveways. The Project will be required to complete any necessary roadway improvements (curb, gutter, and sidewalk, etc.) on Lockwood Street along its frontage. Pedestrian facilities along Lockwood Street will connect the Project to regional and neighborhood commercial and medical services.

#### **Pedestrian and Bicycle Facilities**



Currently there are pedestrian facilities etc.) located (crosswalks/sidewalks along Lockwood Street and Outlet Center Drive in the study-area. The pedestrian facilities connect the Project to the commercial and medical facilities east, west, and south of the Project. Curb gutter and sidewalks are provided on Lockwood Street adjacent to the Project site. The nearest pedestrian crosswalks across Gonzales Road are provided at the Outlet Center Drive signalized intersection. The nearest pedestrian crosswalks across Rose Avenue are

provided at the Lockwood Street signalized intersection. Striped pedestrian crosswalks, ADA ramps and pedestrian call buttons are provided at the Gonzles Road/Outlet Center Drive and Rose Avenue/Lockwood Street intersections. The proposed Project would not have an adverse effect on the existing pedestrian facilities.

The Project site is served by the City of Oxnard Bikeway System. The existing bicycle facilities located in the study-area consist of Class II bike lanes along Gonzales Road, Rose Avenue, Solar Drive, and a portion of Lockwood Street east of the Outlet Center Drive. These Class II bike lanes connect the Project to commercial and employment areas east and west of the Project. The portion of Lockwood Street adjacent to the Project site is identified as a future Class II bike lane facility in the "City of Oxnard Bicycle & Pedestrian Facilities Master Plan". In addition to the on-street facilities, the Project will provide on-site bicycle storage and bike racks. The proposed Project would not have an adverse effect on the existing bicycle facilities.

#### **Transit Facilities**

The City of Oxnard is served by Gold Coast Transit. In the study-area, the Project site is served by several bus routes. The #4A Route (North Oxnard) operates daily providing fixed route bus service on Gonzales Road in the vicinity of the Project site. During the peak commute hours, the #4A Route operates with 45-minute headways. Route (North Oxnard) #4B operates daily providing fixed route bus service on Gonzles Road in the vicinity of the Project site. During the peak commute hours, the #4B Route



operates with 25-minute headways. The #15 Route (Esplanade -El Rio - St. Johns Medical Center) operates daily providing fixed route bus service on Gonzales Road in the vicinity of the Project site. During the peak commute hours, the #15 Route operates with 50-minute headways. The #17 Route (Esplanade - St. Johns Medical Center - Oxnard College) operates daily providing fixed route bus service on Gonzales Road in the vicinity of the Project site. During the peak commute hours, the #17 Route operates with 30- to 45-minute headways. The #19 Route (OTC - 5<sup>th</sup> - Gonzales Road) operates daily providing fixed route bus service on Gonzales Road in the vicinity of the Project site. During the peak commute hours, the #19 Route operates with 60-minute headways. Existing bus stops with benches are located on both sides of Gonzales Road and Rose Avenue, less than ½ a mile from the Project site. The proposed Project has the potential to increase transit ridership and the demand for bus service in the study-area, however these increases would be accommodated by the existing transit services provided.

#### PARKING ANALYSIS

The Lockwood III Apartments Project is proposing to provide 351 on-site parking spaces (including 7 ADA parking spaces, 36 electric vehicle charging stations, 5 ADA electric vehicle charging stations and 1 ADA van electric vehicle charging station). Of the total 351 on-site parking spaces, 39 spaces (11% of total parking) would be compact. The City of Oxnard allows up to 25% of the required parking to be compact. The 39 compact spaces are dispersed through-out the Project site as surface, tuck under and podium parking. The locations of the compact parking spaces would not create an inconvenience, impair safety, on-site vehicular or pedestrian circulation.

#### State Density Bonus Parking Requirement

The Lockwood III Apartments Project is providing a 21% mix of very low- and low-income units, therefore under the State Density Bonus Law (Cal. Gov't Code, Section 65915(p)(3) included in Technical Appendix for reference) and the City of Oxnard Municipal Code, the follow parking ratios apply:

Studio and one bedroom:

1 garage space per unit

Two or more bedrooms:

1.5 garage spaces per unit

Based on these ratios, the parking requirements for the apartment units were calculated as shown in Table 10.

Table 10
Density Bonus Parking Requirements

Dwelling Type	Number of Units	Parking Ratio	Required Parking
1 Bedroom Apartment	24 units	1 space/unit	24 spaces
1 Bedroom Apartment	78 units	1 space/unit	78 spaces
2 Bedroom Apartment	108 units	1.5 spaces/unit	162 spaces
3 Bedroom Apartment	24 units	1.5 spaces/unit	36 spaces
	300 spaces		

Based on the Density Bonus Law, 300 parking spaces are required for the 234 apartment units in the Lockwood III Apartments Project. The 351 on-site parking spaces proposed for the Project would satisfy the parking requirement and provide 51 reserve parking spaces.

#### Project Peak Parking Demands

Peak parking demand estimates were also developed for the Project. Based on the empirical parking demand rates presented in the ITE, <u>Parking Generation</u>, 5<sup>th</sup> Edition. ITE parking demand rates (85<sup>th</sup> Percentile) for Multifamily Housing - Mid Rise (Land Use Code #221) and Affordable Housing - Income Limits (Land Use Code #223) was used to estimate the peak parking demand for the Project based on the number of apartment units. Table 11 summarizes the results of the peak parking demand analysis based on ITE parking demand data.

Table 11
ITE Peak Parking Demand

Land Use	Units	Parking Rate	<b>Parking Demand</b>	<b>Parking Provided</b>
Apartments - Market Rate	201 units	1.47 Vehicles/Unit	295 Spaces	
Apartments - Affordable	33 units	1.33 Vehicles/Unit	44 Spaces	351 Spaces
·		Total Parking:	339 Spaces	

Based on ITE rates, the weekday peak parking demand estimates for the Lockwood III Apartments Project is 339 parking spaces. The 351 on-site parking spaces proposed for the Project would satisfy the peak parking demand and provide 12 reserve parking spaces.

Based on State Density Bonus Law and ITE empirical peak parking demand data, the 351 on-site parking spaces proposed by the Project are adequate.

#### **CONSTRUCTION TRAFFIC ANALYSIS**

Project grading and building construction is estimated to occur over 22 months. Grading is estimated to occur over a 2-month period. The average number of workers on the site for the grading and construction duration is 10 - 30 workers. The workers make one trip onto the job site at 7:00 AM, and one trip off the job site at 3:00 PM. The vast majority of workers bring their own lunch or eat from a food truck that comes to the site, thus never leaving the site during work hours. Construction workers would travel to the site in private vehicles. If adequate parking areas are not available onsite, off-site parking would need to be obtained.

Grading and building construction of the Project would also require the delivery of construction equipment and materials to the site. Adequate staging and storage areas would need to be provided on-site to accommodate construction equipment and the delivery and storage of materials. The average heavy work trucks on site every day is 2. During the grading portion of the Project (first stage of construction) this number will go up slightly. The Project will have to have dirt hauled offsite, so during this process there will be 3 heavy trucks hauling dirt to an offsite location. Each truck will make an estimated 1 - 2 trips per day.

Construction of project-related improvements along Lockwood Street and Outlet Center Dive would have the potential to disrupt traffic flows within the corridor. Certain construction activities - such as roadway improvements, utility relocation or extensions, and drainage facility construction - could require temporary lane closures, which would in turn temporarily reduce existing street capacity. Such impacts would be short-term in duration. Potential sidewalk and lane closures could affect pedestrian and bicycle flows and therefore should be managed to minimize potential impacts. Traffic control plans would be required to mitigate these potential construction impacts (see Construction Mitigation Measures).

#### **VEHICLE MILES TRAVELED ANALYSIS**

The State of California, in compliance with Senate Bill 743, has developed a new set of CEQA guidelines and thresholds for transportation impacts that are based on a Vehicle Miles Traveled (VMT) metric rather than a Level of Service (LOS) metric. The State's Natural Resource Agency Updated Guidelines for the Implementation of the CEQA adopted in 2018, have designated VMT as the most appropriate measure of transportation impacts. "Vehicle miles traveled" refers to the amount and distance of automobile travel attributable to a project. Other relevant considerations may include the effects of the project on transit and non-motorized travel. For land use projects, vehicle miles traveled exceeding an applicable threshold of significance may indicate a significant impact.

#### VMT Thresholds

Local agencies have discretion to develop and adopt their own thresholds or rely on thresholds recommended by other agencies. Since the City of Oxnard has not yet adopted VMT impact criteria, the VMT analyses prepared for the Project was developed using VMT data presented in the recently updated Ventura County Transportation Commission (VCTC) traffic model for Ventura County and the following VMT thresholds published by the State.

The California Governor's Office of Planning and Research (OPR) published a technical advisory that includes recommendations regarding assessment of VMT, thresholds of significance, and mitigation measures.<sup>3</sup> The recommended VMT impact threshold for residential projects is as follows:

"A proposed project exceeding a level of 15 percent below existing VMT per capita may indicate a significant transportation impact. "

#### VMT Analysis

The VCTC traffic model provides home based VMT per capita data for the City of Oxnard as well as the various Traffic Analysis Zones (TAZs) within the City, including the TAZ that encompasses the Project site (TAZ included in the Technical Appendix). Traffic model data was used to establish the home-based VMT per capita thresholds for the City of Oxnard and to estimate the home-based VMT per capita for the Project. Table 12 shows the existing home-based VMT per capita for the City of Oxnard, the VMT threshold (15% below existing home-based VMT per capita), and the Project's home-based VMT per capita based on the VCTC traffic model data (model data attached).

Table 12 Lockwood III Apartments Per Capita VMT Summary

City of Oxnard VMT <sup>(a)</sup>	VMT Impact Threshold <sup>(b)</sup>	Project VMT Estimate(c)	Impact?
14.80 per capita	10.69 per capita	0.97 per capita	No

<sup>(</sup>a) City of Oxnard home-based VMT per capita based on VCTC traffic model.

As shown, the existing city-wide home-based VMT in the City of Oxnard is 14.80 VMT per capita. Thus, the VMT threshold is 10.69 VMT per capita (15% below existing VMT: 14.80 x 0.85 = 10.69). The VCTC model shows that the homes within the Project TAZ generate 0.97 VMT per capita, which is below the 10.69 VMT per capita impact threshold. Thus, the Project would not have a potentially significant VMT impact.

<sup>(</sup>b) VMT Threshold is a 15% reduction from City VMT (14.80  $\times$  0.85 = 10.69).

<sup>(</sup>c) Project home-based VMT per capita estimate based on VCTC model traffic analysis zones.

Technical Advisory on Evaluating Transportation Impacts in CEQA, Governor's Office of Planning and Research, December 2018.

#### REFERENCES AND PERSONS CONTACTED

#### **Associated Transportation Engineers**

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#### **Persons Contacted**

Earnel Bihis, City of Oxnard

#### References

Trip Generation, Institute of Transportation Engineers, 11th Edition, 2020.

Technical Advisory on Evaluating Transportation Impacts in CEQA, Governor's Office of Planning and Research, December 2018.

City of Oxnard Bicycle & Pedestrian Facilities Máster Plan, City of Oxnard, February 2011.

<u>Traffic and Circulation Analysis for the Factory Outlet Center Project</u>, Associated Transportation Engineers, August 1992.

Sakioka Farms Specific Plan Environment Impact Report Traffic Study, Austin-Foust Associates, Inc. February 2010.

Mission Produce Office Project Traffic and Circulation Study, Associated Transportation Engineers, December 2018.

#### **TECHNICAL APPENDIX**

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Reference 2 - U.S. Highway 101 Southbound Ramps/Rice Avenue

Reference 3 - Rice Avenue/Gonzales Road

Reference 4 - U.S. Highway 101 Northbound Ramps/Rose Avenue

Reference 5 - U.S. Highway 101 Southbound Ramps/Rose Avenue

Reference 6 - Rose Avenue/Lockwood Street

Reference 7 - Rose Avenue/Gonzales Road

Reference 8 - Gonzales Road/Williams Drive

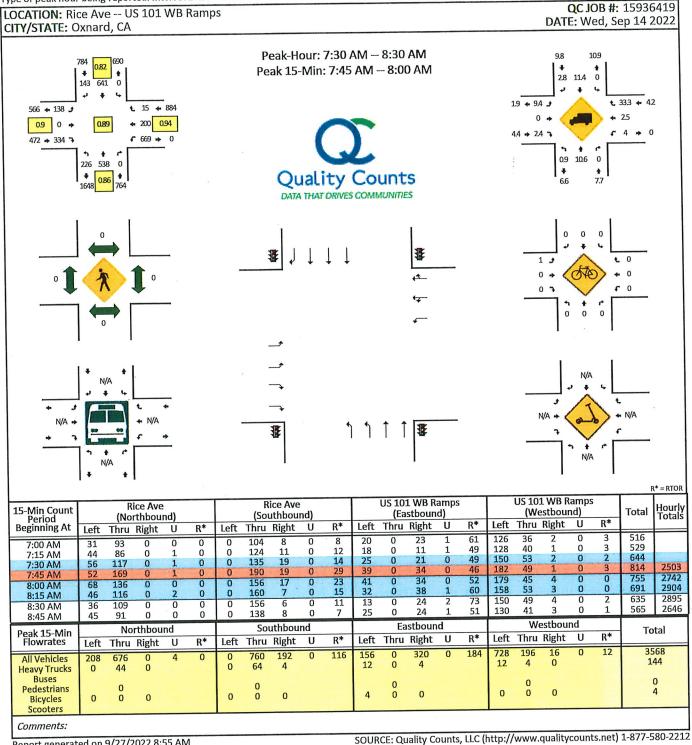
Reference 9 - Gonzales Road/Lombard Street

Reference 10 - Gonzales Road/Solar Drive

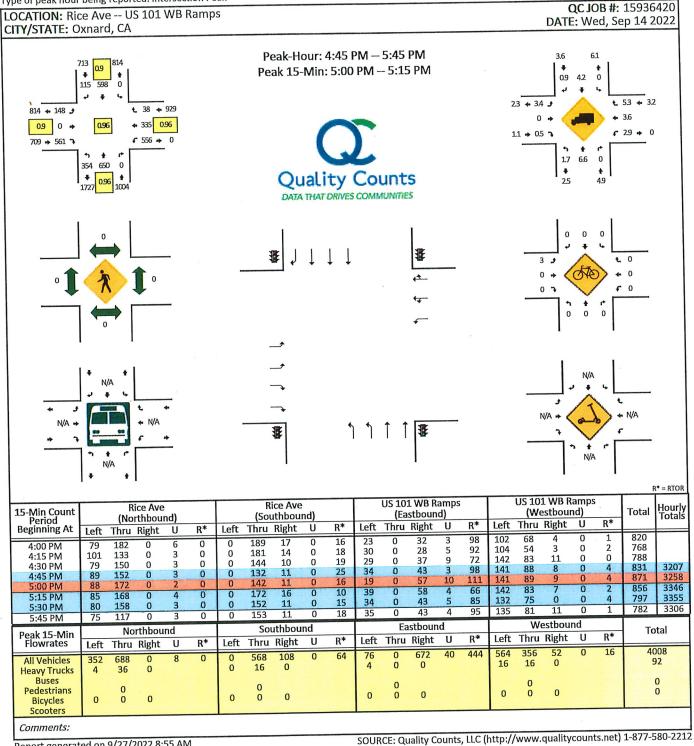
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VENTURA COUNTY TRANSPORTATION COMMISSION TRAVEL DEMAND MODEL TAZ

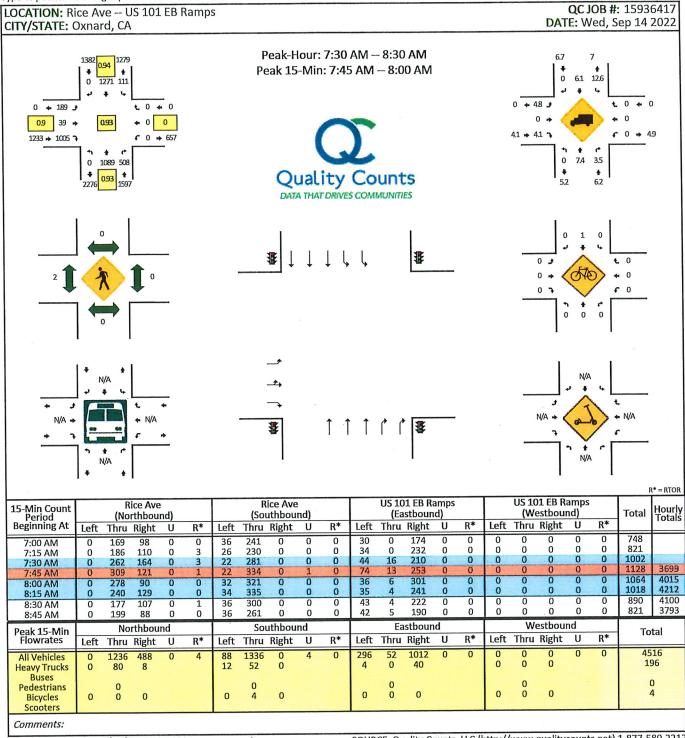
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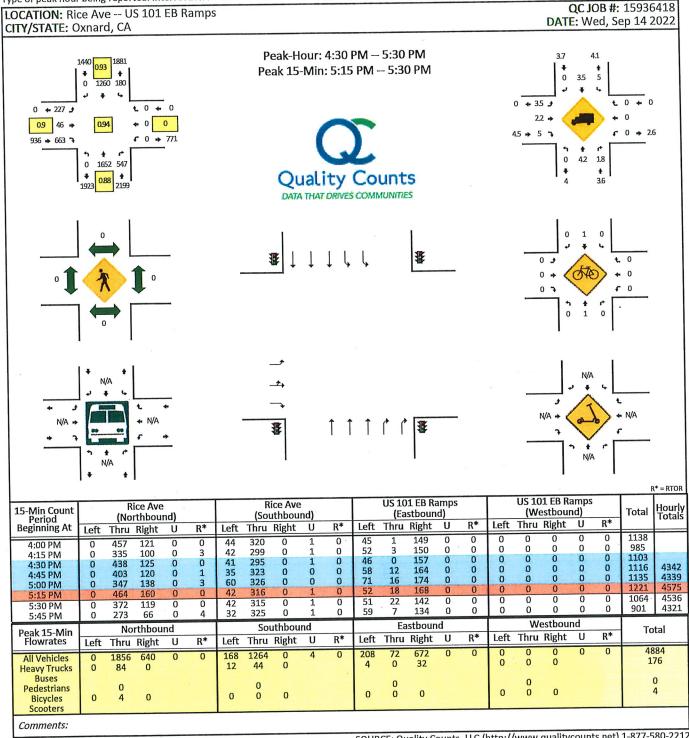


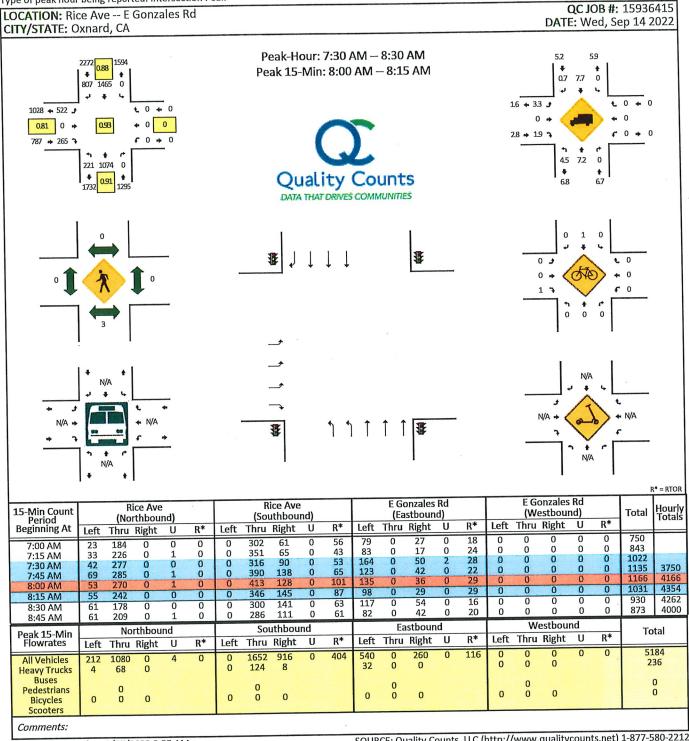
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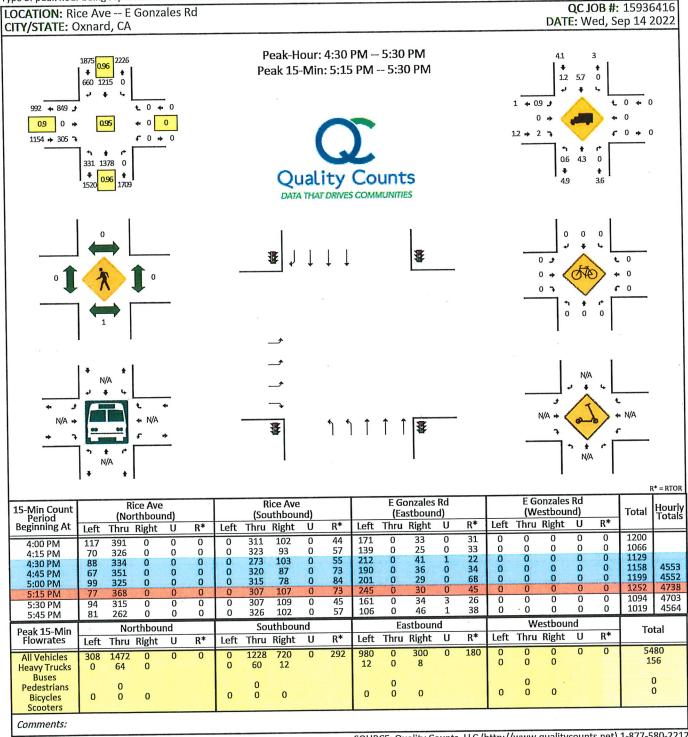


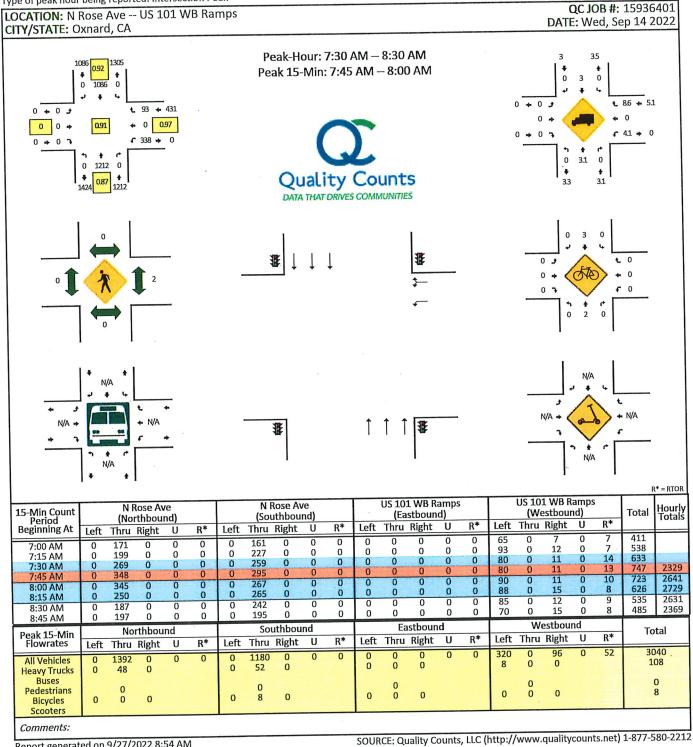
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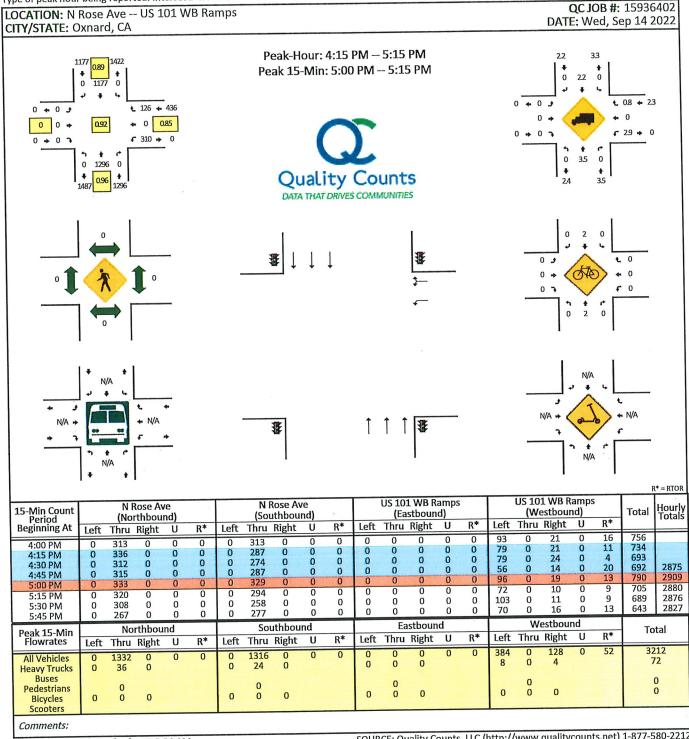
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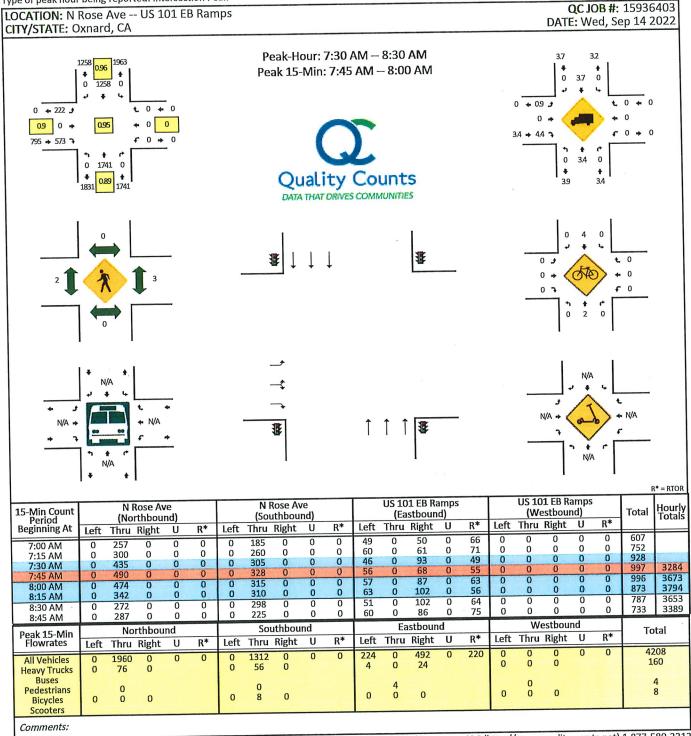


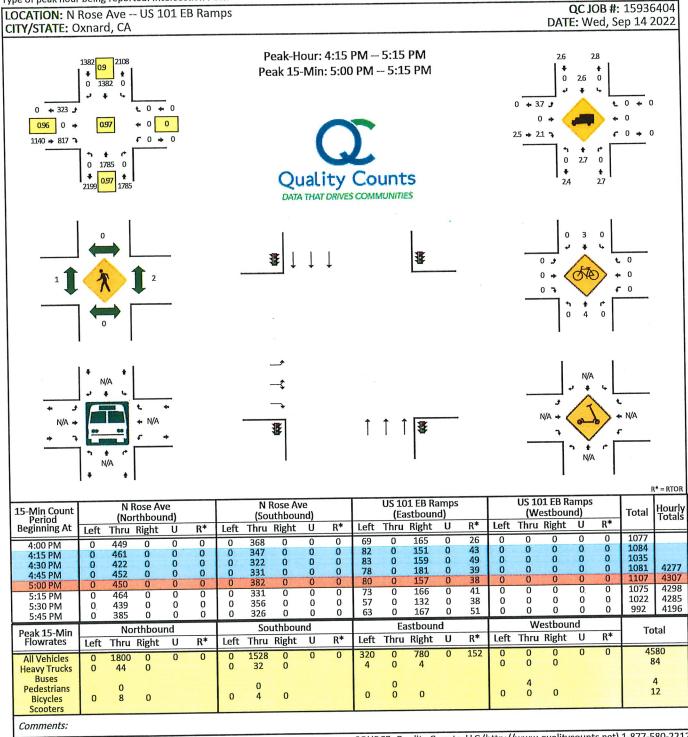


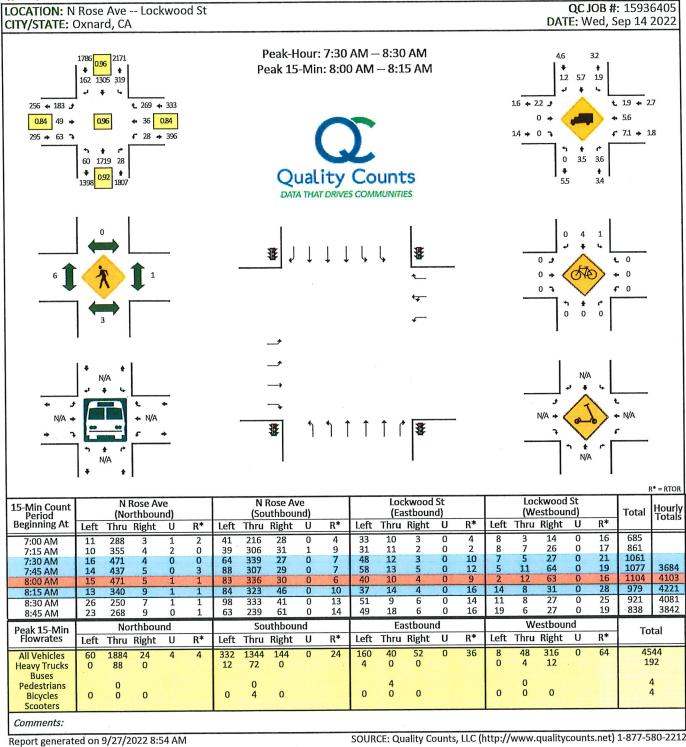


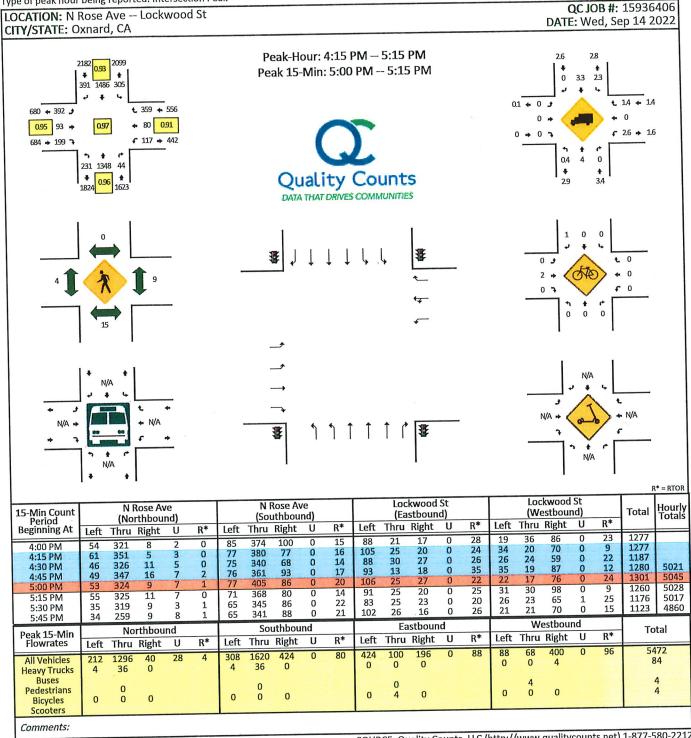


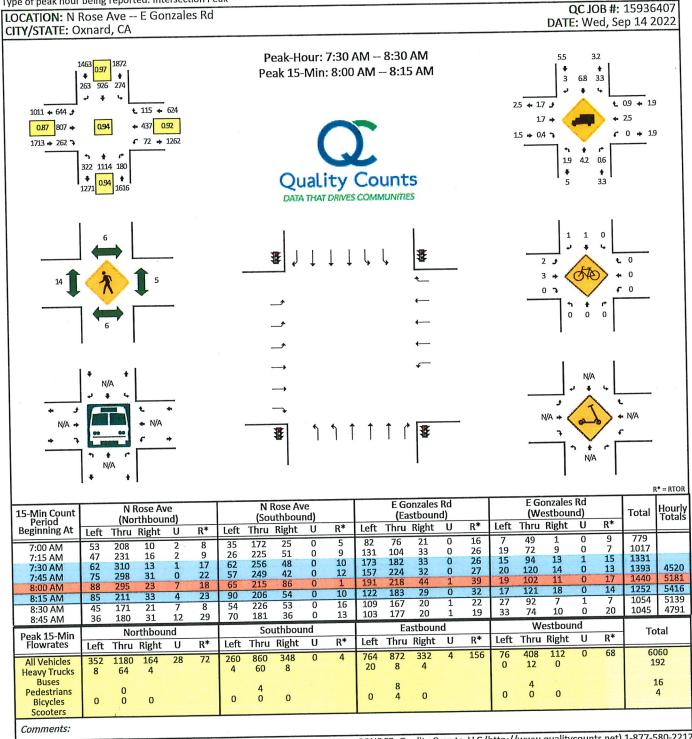


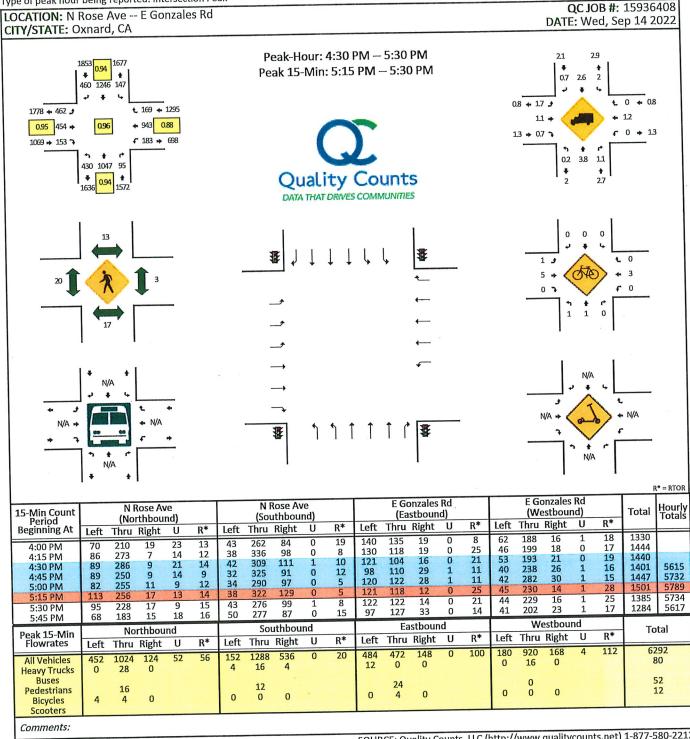


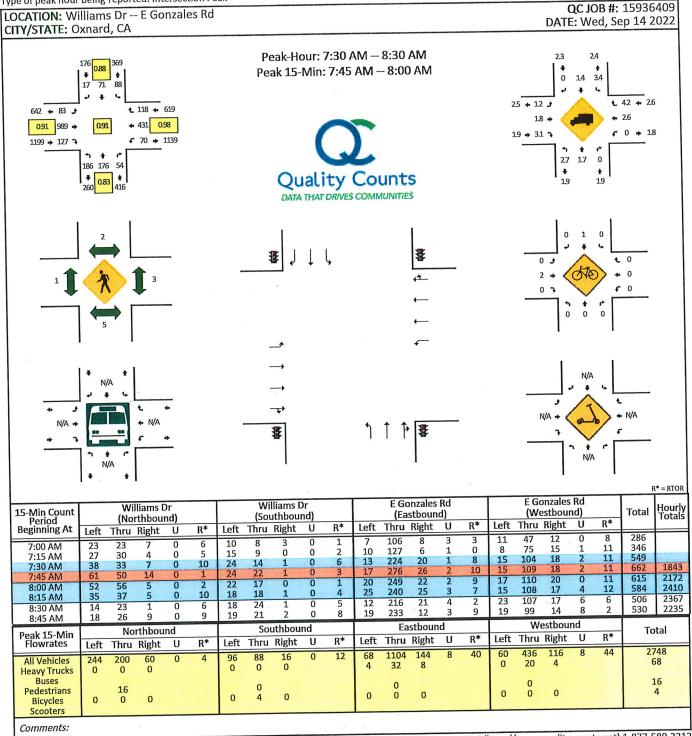


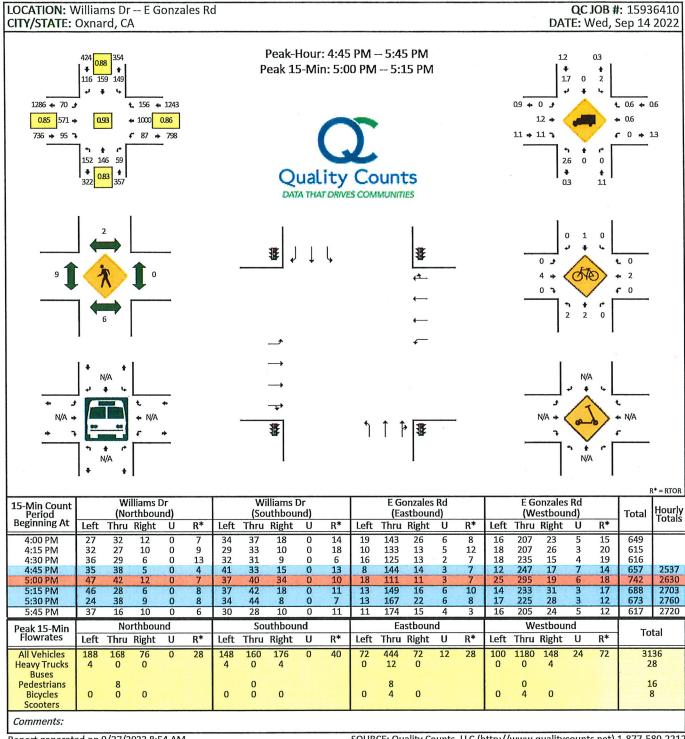


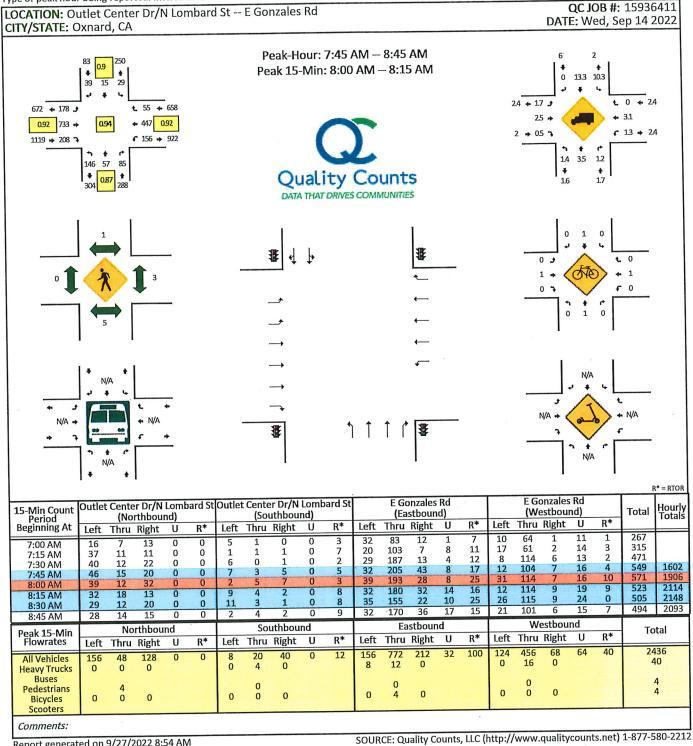


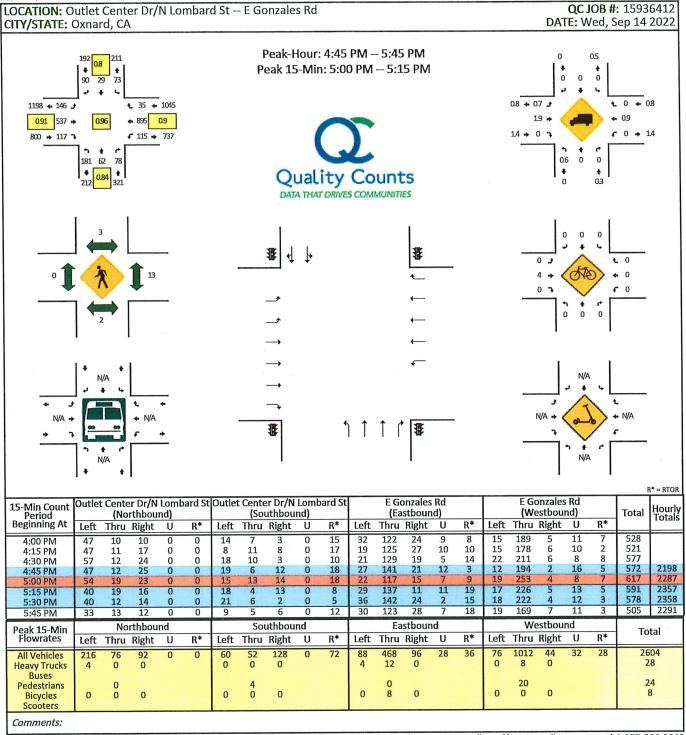


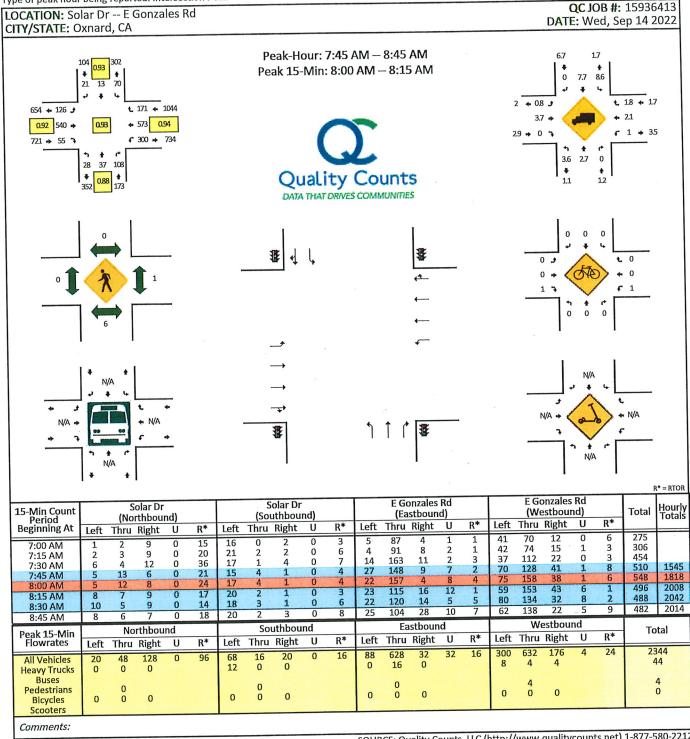


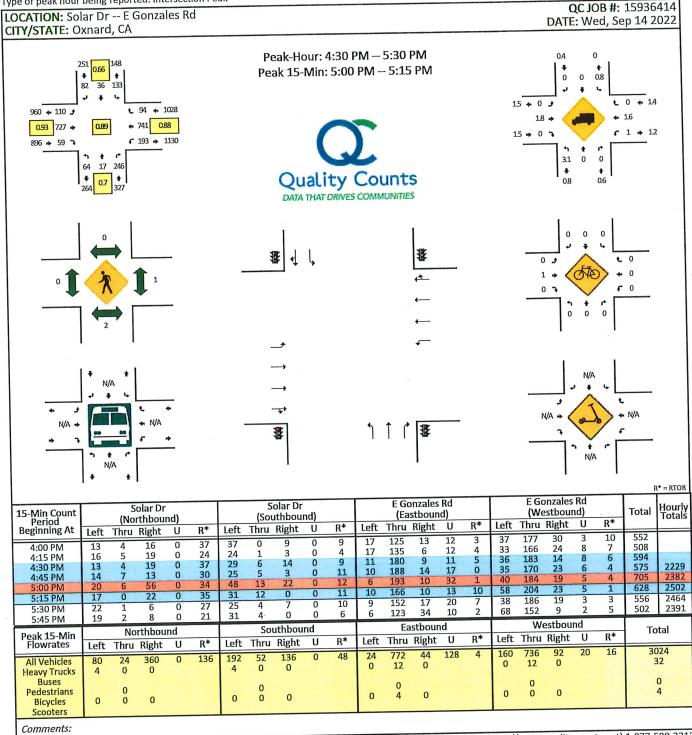












INTERSECTION LEVEL OF SERVICE CRITERIA/DEFINITIONS

# DISCUSSION OF INTERSECTION CAPACITY UTILIZATION (ICU)

The ability of a roadway to carry traffic is referred to as capacity. The capacity is usually less at intersections because traffic flows continuously between them and only during the green phase at them. Capacity at intersections is best defined in terms of vehicles per lane per hour of green. The technique used to compare the volumes and capacity of an intersection is known as Intersection Capacity Utilization (ICU). ICU or volume-to-capacity ratio, usually expressed as a percentage, is the proportion of an hour required to provide sufficient capacity to accommodate all intersection traffic if all approaches operate at capacity. If an intersection is operating at 80 percent of capacity, then 20 percent of the signal cycle is not used.

The ICU calculation assumes that an intersection is signalized and that the signal is ideally timed. Although calculating ICU for an unsignalized intersection is invalid, the presumption is that a signal can be installed and the calculation shows whether the geometrics are capable of accommodating the expected volumes. It is possible to have an ICU well below 100 percent, yet have severe traffic congestion. This would occur if one or more movements is not getting sufficient time to satisfy its demand, and excess time exists on other movements. This is an operational problem which should be addressed.

Capacity is often defined in terms of roadway width. However, standard lanes have approximately the same capacity whether they are 11 or 14 feet wide. Data collected by Kunzman Associates indicates a typical lane, whether a through-lane or a left-turn lane, has a capacity of approximately 1,700 vehicles per hour, with nearly all locations showing a capacity greater than 1,600 vehicles per hour per lane. This finding is published in the August, 1978 issue of <a href="ITE Journal">ITE Journal</a> in the article entitled, "Another Look at Signalized Intersection Capacity" by William Kunzman. For this study, a capacity of 1,600 vehicles per hour per lane will be assumed for left-turn, through, and right-turn lanes as per City policy.

The yellow time can either be assumed to be completely used and no penalty applied, or it can be assumed to be only partially usable. Total yellow time accounts for less than 10 percent of a cycle, and a penalty of up to five percent is reasonable. On the other hand, during peak hour traffic operation, the yellow times are nearly completely used. In this study, no penalty will be applied for the yellow because the capacities have been assumed to be only 1,600 vehicles per hour per lane when in general they are 1,700-1,800 vehicles per hour per lane.

The ICU technique is an ideal tool to quantify existing as well as future intersection operations. The impact of adding a lane can be quickly determined by examining the effect the lane has on the intersection capacity utilization.

Source: Oxnard Airport Business Park Traffic Study, Kunzman Assoc., City of Oxnard, 1985.

# Signalized Intersection Level of Service Definitions

LOS	Delaya	V/C Ratio	Definition
A	< 10.0	< 0.60	Progression is extremely favorable. Most vehicles arrive during the green phase. Many vehicles do not stop at all.
В	10.1 - 20.0	0.61 - 0.70	Good progression, short cycle lengths, or both. More vehicles stop than with LOS A, causing higher levels of delay.
С	20.1 - 35.0	0.71 - 0.80	Only fair progression, longer cycle lengths, or both, result in higher cycle lengths. Cycle lengths may fail to serve queued vehicles, and overflow occurs. Number of vehicles stopped is significant, though many still pass through intersection without stopping.
D	35.1 - 55.0	0.81 - 0.90	Congestion becomes more noticeable. Unfavorable progression, long cycle lengths and high v/c ratios result in longer delays. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	55.1 - 80.0	0.91 - 1.00	High delay values indicate poor progression, long cycle lengths and high v/c ratios. Individual cycle failures are frequent
F	> 80.0	> 1.00	Considered unacceptable for most drivers, this level occurs when arrival flow rates exceed the capacity of lane groups, resulting in many individual cycle failures. Poor progression and long cycle lengths may also contribute to high delay levels.

<sup>&</sup>lt;sup>a</sup> Average control delay per vehicle in seconds.

# **Unsignalized Intersection Level of Service Definitions**

The HCM¹ uses *control delay* to determine the level of service at unsignalized intersections. Control delay is the difference between the travel time actually experienced at the control device and the travel time that would occur in the absence of the traffic control device. Control delay includes deceleration from free flow speed, queue move-up time, stopped delay and acceleration back to free flow speed.

LOS	Control Delay Seconds per Vehicle
А	< 10.0
В	10.1 - 15.0
С	15.1 - 25.0
D	25.1 - 35.0
Е	35.1 - 50.0
F	> 50.0

<sup>&</sup>lt;sup>1</sup> Highway Capacity Manual, National Research Board, 2000

# INTERSECTION LOS CALCULATION WORKSHEETS

Reference 1 - U.S. Highway 101 Northbound Ramps/Rice Avenue

Reference 2 - U.S. Highway 101 Southbound Ramps/Rice Avenue

Reference 3 - Rice Avenue/Gonzales Road

Reference 4 - U.S. Highway 101 Northbound Ramps/Rose Avenue

Reference 5 - U.S. Highway 101 Southbound Ramps/Rose Avenue

Reference 6 - Rose Avenue/Lockwood Street

Reference 7 - Rose Avenue/Gonzales Road

Reference 8 - Gonzales Road/Williams Drive

Reference 9 - Gonzales Road/Lombard Street

Reference 10 - Gonzales Road/Solar Drive

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

RICE AVENUE

E/W STREET:

U.S.HIGHWAY 101 NORTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

					RAFFIC						CT DOLLVIE		 	_
	NOR	TH BOL	JND	SOL	JTH BO	UND	EAST	L BOU!	ND	WE	ST BOUND	,		
VOLUMES	1	Т	R	L	T	R	L	T	R	L	T	R		
(A) EXISTING: (B) PROJECT-ADDED: (C) CUMULATIVE:	226 0 264	538 4 706	0 0 0	0 0 0	641 1 842	143 0 157	138 0 196	0 0 0	334 0 434	669 4 771	200 0 225	15 0 23		

### **GEOMETRICS**

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 1 AM

LANE GEOMETRICS

LL TT

TTT R

LL RR

L LT TR

## TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SER	VICE CALCULATION	NS					
MOVE-	# OF			SCEN	NARIO V	OLUMES		3	SCENARIO V	//C RATIOS	1	1
MENTS	LANES	CAPACITY	1	2	3	4	1	2	3	4		-
			226	226	264	264	0.071 *	0.071 *	0.083 *	0.083 *		1
NBL	2	3200	538	542	706	710	0.168	0.169	0.221	0.222		
NBT	2	3200 0	0	0	0	0	-	-	-			1
NBR	0	U		Ü	Ü	Ü						
CDI	0	0	0	0	0	0	-	-	-	-		
SBL	3	4800	641	642	842	843	0.134 *	0.134 *	0.175 *	0.176 *		
SBT	1	1600	143	143	157	157	0.089	0.089	0.098	0.098		
SBR	1	1000										
EBL	2	3200	138	138	196	196	0.043	0.043	0.061	0.061		
EBT	0	0	0	0	0	0	-	-	-	-		
EBR	2	3200	334	334	434	434	0.104 *	0.104 *	0.136 *	0.136 *		
LDK	-										*	
WBL	0	0	669	673	771	775	-	-	-	-		
WBT	3	4800	200	200	225	225	0.184 *	0.185 *	0.212 *	0.213 *		
WBR	0	0	15	15	23	23	-	-	-	-		
										0.00		
						LOST TIME:	0.00	0.00	0.00	0.00		
		TO	TAL INTE	RSECTIO	N CAPA	CITY UTILIZATION:	0.49	0.49	0.61	0.61		
1						EL OF SERVICE:	A	A	В	В		
				representation and	en established							

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

RICE AVENUE

E/W STREET:

U.S.HIGHWAY 101 NORTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

				Т	RAFFIC	VOLU	ME SUN	MARY	′				
	NOR	TH BO	JND	SOL	JTH BO	JND	EAS	r Bout	ND	WE	ST BOUNE	)	
VOLUMES	L	Т	R	L	T	R	L	T	R	L	T	R	
(A) EXISTING:	354	650	0	0	598	115	148	0	561 0	556 13	335 0	38 0	
<ul><li>(B) PROJECT-ADDED:</li><li>(C) CUMULATIVE:</li></ul>	0 480	815	0	0	713	133	164	0	661	661	348	40	

# **GEOMETRICS**

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 1 PM

LANE GEOMETRICS

LL RR

LL TT

TTT R

L LT TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SER	RVICE CALCULATIO	NS					
MOVE-	# OF			SCE	NARIO V	'OLUMES		2	SCENARIO V	//C RATIOS		1
MENTS	LANES	CAPACITY	1	2	3	4	1	2	3	4		
		3200	354	354	480	480	0.111 *	0.111 *	0.150 *	0.150 *		1
NBL	2	3200	650	652	815	817	0.203	0.204	0.255	0.255	-	1
NBT	2	0	0	0	0	0	-	-	-	-		
NBR	0	U	U	Ü	Ü	-						1
SBL	0	0	0	. 0	0	0	-	-	-	-		
SBT	3	4800	598	601	713	716	0.125 *	0.125 *	0.149 *	0.149 *		
SBR	1	1600	115	115	133	133	0.072	0.072	0.083	0.083		
SDK	'	1000	9 0000									
EBL	2	3200	148	148	164	164	0.046	0.046	0.051	0.051		
EBT	0	0	0	0	0	0	-	-	-	-		
EBR ·	2	3200	561	561	661	661	0.175 *	0.175 *	0.207 *	0.207 *		
EBK		*										
WBL	0	0	556	569	661	674	-	-	-	-		
WBT	3	4800	335	335	348	348	0.194 *	0.196 *	0.219 *	0.221 *		81
WBR	0	0 .	38	38	40	40	-	-	- '	-		
								0.00	0.00	0.00		
						LOST TIME:	0.00	0.00	0.00	0.00		
1												
l		TO	TAL INTE	RSECTIO	N CAPA	CITY UTILIZATION:	0.61	0.61	0.73	0.73		
				SCENA	RIO LEVE	EL OF SERVICE:	В	В	С	C		
					APR DIVINI							

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

RICE AVENUE

E/W STREET:

U.S.HIGHWAY 101 SOUTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

				TF	RAFFIC	VOLU	ME SUN	MARY	,				 _
	NOF	RTH BO	JND	SOU	TH BOL	JND	EAST	BOU	ND	WE	est bound	)	
VOLUMES	L	Т	R	L	Τ	R	L	T	R	L	T	R	_
(A) EXISTING:	0	1089	508	111	1271	0	189	39	1005	0	0	0	
(B) PROJECT-ADDED:	0	4	15	0	7	0	0	0	0	0	0	0	
(C) CUMULATIVE:	0	1550	572	124	1675	0	376	40	1205	0	0	0	

GEOMETRICS

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 2 AM

LANE GEOMETRICS

TTT RR

TTT LL

LLTR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SEI	RVICE CA	LCULATION	NS				 LEVEL OF SERVICE CALCULATIONS  SCENARIO VOLUMES SCENARIO V/C RATIOS													
MOVE-	# OF			SCE	NARIO \	OLUMES			2	SCENARIO V	//C RATIOS														
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4														
MENTS				0	0	0		_	_	_	-														
NBL	0	0	0	0				0.227 *	0.228 *	0.323 *	0.324 *														
NBT	3	4800	1089	1093	1550	1554		0.159	0.163	0.179	0.183														
NBR	2	3200	508	523	572	587		0.159	0.103	0.173	0.103														
						404		0.035 *	0.035 *	0.039 *	0.039 *														
SBL	2	3200	111	111	124	124			0.266	0.349	0.350														
SBT	3	4800	1271	1278	1675	1682		0.265	0.266	0.349	0.550														
SBR	0	0	0	0	0	0		-	-	-	-														
EBL	0	0	189	189	376	376		-	-	-	-														
EBT	3	4800	39	39	40	40		0.257 *	0.257 *	0.338 *	0.338 *														
EBR	0	0	1005	1005	1205	1205		-	-	-	-														
LDK																									
WBL	0	0	0	0	0	0		-	-	-	-														
WBT	0	0	0	0	0	0	-	-	-	-	-														
WBR	0	0	0	0	0	0		-	-	-	-														
VVDK	U							-																	
						LOST	TIME:	0.00	0.00	0.00	0.00														
			0.53	0.52	0.70	0.70																			
		TC	TAL INTE				IZATION:	0.52			B														
1				SCENA	RIO LEV	'EL OF SER'	VICE:	A	A	В	В														
	466		No. 100 100 100	****		10 10 10 10 10 10 10 10 10 10 10 10 10 1			The state of the s																

**NOTES:** 

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

RICE AVENUE

E/W STREET:

U.S.HIGHWAY 101 SOUTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

				TI	RAFFIC	VOLU	ME SUN	<i>M</i> ARY	•				 
	NOF	RTH BOL	JND	SOU	TH BOL	JND	EAS <sup>-</sup>	r Bout	۷D	WE	ST BOUNI	)	
VOLUMES	L	Т	R	L	T	R	L	T	R	L	T	R	
(A) EXISTING:	0	1652	547	180	1260 16	0	227	46 0	663 0	0	0	0	
<ul><li>(B) PROJECT-ADDED:</li><li>(C) CUMULATIVE:</li></ul>	0	2 2706	858	190	1580	0	245	50	863	0	0	0	

**GEOMETRICS** 

TRAFFIC SCENARIOS

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 2 PM

LANE GEOMETRICS

TTT RR

L TTT

L LT R

SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES (A + B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

	LEVEL OF SERVICE CALCULATIONS  SCENARIO VOLLIMES SCENARIO V/C RATIOS													
MOVE-	# OF			SCE	NARIO \	VOLUMES		9	SCENARIO V	//C RATIOS				
MENTS	LANES	CAPACITY	1	2	3	4	1	2	3	4				
		0	0	0	0	0	_	_		_				
NBL	0	0		1654	2706	2708	0.344 *	0.345 *	0.564 *	0.564 *				
NBT	3	4800	1652		858	866	0.171	0.173	0.268	0.271				
NBR	2	3200	547	555	030	000	0.171	0.175	0,200					
		2200	180	180	190	190	0.056 *	0.056 *	0.059 *	0.059 *				
SBL	2	3200	1260	1276	1580	1596	0.263	0.266	0.329	0.333				
SBT	3	4800			0		0.200			_				
SBR	0	0	0	0	U	. 0								
			007	207	2.45	245			_					
EBL	0	0	227	227	245	245	0.154 *	0.154 *	0.187 *	0.187 *				
EBT	3	4800	46	46	50	50	0.134	0.134	0.107	0.107				
EBR	0	0	464	464	604	604	-	-	-	_				
F						200								
WBL	0	0	0	. 0	0	0	-	-	-	-				
WBT	0	0	0	0	0	0	-	-	-	-				
WBR	0	0	0	0	0	0	1	- ,	-	-				
						LOST TIME:	0.00	0.00	0.00	0.00				
		TO	TAI INTE	RSECTIO	N CAPA	CITY UTILIZATION:	0.55	0.56	0.81	0.81				
		10	TAL HAIL			EL OF SERVICE:	A	A	D	D				
				JCLIAN	INO LLY	LL C. OLIVIOLI								

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: SEPTEMBER 14, 2022

TIME PERIOD: AM PEAK HOUR

N/S STREET:

RICE AVENUE

E/W STREET:

GONZALEZ ROAD

CONTROL TYPE: SIGNAL

	Sandiske (austria)			Т	RAFFIC	VOLU	ME SUN	<b>MARY</b>	,				
	NOR	TH BO	JND	SOL	JTH BOI	JND	EAS	r Boui	ND	WE	st bouni	O	
VOLUMES	L	Т	R	L	T	R	L	T	R	L	T	R	
(A) EXISTING: (B) PROJECT-ADDED:	221 1	1074 0	0 0	0	1465 0	807	522 19	0 0	265 4	0 0	0 0	0	

REF: 3 AM

		GEOME.	TRICS		
LANE GEOMETRICS	NORTH BOUND LL TTTR	SOUTH BOUND L TTT R	EAST BOUND LL LT R	WEST BOUND LTR	

#### TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A + B)

				LEVEL	OF SERV	ICE CALCULATION	NS					
MOVE-	# OF			SCEN	IARIO VO	LUMES		9	CENARIO V	/C RATIOS		
MENTS	LANES	CAPACITY	1	2	3	4	1	2	3	4		
NBL	2 3	3200 4800	221 1074	222 1074			0.069 * 0.224	0.069 * 0.224				y
NBT NBR	0	0	0	0			-	-				
SBL SBT SBR	1 3 1	1600 4800 1600	0 1465 807	0 1465 814			0.000 0.305 * 0.504	0.000 0.305 * 0.509			÷	
EBL EBT EBR	0 4 0	0 6400 0	522 0 265	541 0 269			0.123 *	- 0.127 * -	·			
WBL WBT WBR	0 1 0	0 1600 0	0 0	0 0 0			0.000 *	- 0.000 * -				
		TC	OTAL INTE			LOST TIME: TY UTILIZATION: OF SERVICE:	0.00 0.50 A	0.00 0.50 A				

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

VOLUMES

RICE AVENUE GONZALEZ ROAD

E/W STREET: CONTROL TYPE:

SIGNAL

SOUTH BOUND EAST BOUND WEST BOUND					MMARY	ME SU	VOLUM	RAFFIC	T	
		EST BOUND	W	4D						
L T R L T R	R	T	L	R	T	L	R	Τ	L	

(A) EXISTING: (B) PROJECT-ADDED:

T 305 0 331 1378 0 0 1215 660 849 2 0 0 16 10 0 0 0 3

GEOMETRICS

LANE GEOMETRICS

NORTH BOUND LL TTTR

NORTH BOUND

SOUTH BOUND L TTT R

EAST BOUND LL LT R

**WEST BOUND** LTR

REF: 3 PM

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

				LEVE	L OF SERV	ICE CALCUL	ATION	18					
MOVE-	# OF			SCE	NARIO VO	DLUMES			9	SCENARIO V	C RATIOS		
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4		
NBL	2	3200	331	334				0.103 * 0.287	0.104 * 0.287				
NBT NBR	3	4800 0	1378 0	1378 0				-	-				
SBL SBT SBR	1 3	1600 4800 1600	0 1215 660	0 1215 676				0.000 0.253 * 0.413	0.000 0.253 * 0.423				
EBL EBT EBR	0 4 0	0 6400 0	849 0 305	859 0 307	×			- 0.180 * -	- 0.182 * -				
WBL WBT WBR	0 1 0	0 1600 0	0 0 1	0 0 1				0.001 *	- 0.001 * -			·	Ų
						LOST TIME:		0.00	0.00			ε	23

0.54

Α

0.54

Α

TOTAL INTERSECTION CAPACITY UTILIZATION:

SCENARIO LEVEL OF SERVICE:

NOTES:

10/12/22

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

MAY 2, 2018

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

RICE AVENUE

E/W STREET:

GONZALEZ ROAD

CONTROL TYPE:

SIGNAL

CONTROL TYPE:	SIGIVAL							35-45-		av resignation is				
	In Alberta Wall American				TF	RAFFIC	VOLUM	ME SUM	MARY					_
		NOR	TH BOL	JND	SOU	TH BOL	JND	EAS	T BOUN	ID	WES	ST BOUND	)	
		1	Т	R	ı	T	R	L	T	R	L	T	R	
VOLUMES		L			-									
(B) PROJECT-ADDEE (C) CUMULATIVE:	):	1 284	0 1409	0 405	0 190	0 2127	3 1017	6 565	0 1075	1 498	0 210	0 250	0 245	

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

WEST BOUND

REF: 3 AM

IMPROVED LANE GEOMETRICS

LL TTTT R

LL TTTT R

LL TTTT R

LLL TTT R

#### TRAFFIC SCENARIOS

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

- Annual Control of the Control of t			LEVEL OF SEI	RVICE CA	LCULATION	IS				 
	# OF		SCENARIO \	OLUMES			9	SCENARIO V	//C RATIOS	
MOVE-		CAPACITY	1 2 3	4		1	2	3	4 .	
MENTS	LANES	CAFACITI						0.089 *	0.089 *	
NBL	.2	3200	284	285				0.009	0.220	
NBT	4	6400	1409	1409				0.220	0.253	
NBR	1	1600	405	405				0.255	0.233	
SBL SBT SBR	2 4 1	3200 6400 1600	190 2127 1017	190 2127 1020				0.059 0.332 * 0.636	0.059 0.332 * 0.638	
EBL EBT EBR WBL WBT	2 4 1 3 3	3200 6400 1600 4800 4800	565 1075 498 210 250	571 1075 499 210 250	•			0.177 * 0.168 0.311  0.044 0.052	0.178 * 0.168 0.312  0.044 0.052 0.153 *	
WBR	1	1600	245	245	9			0.153 *	0.133	
		TO	TAL INTERSECTION CAPA SCENARIO LEV		IZATION:			0.00 0.75 C	0.00 0.75 C	

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

VOLUMES

RICE AVENUE

E/W STREET:

GONZALEZ ROAD

CONTROL TYPE:

SIGNAL

er Campaner E.			T	RAFFIC	VOLU	ME SUI	MMARY	′	No. Co. Co. Co. Co. Co. Co. Co. Co. Co. C			
NOF	RTH BO	UND	SOL	JTH BO	UND	EAS	T BOUI	ND	WE	ST BOUN	D	
L	T	R	L	T	R	L	T	R	L	T	R	

(B) PROJECT-ADDED:

0

190

5 975 570

1 337

0 710

0 1091 0

REF: 3 PM

520

(C) CUMULATIVE:

419 2480

1

90

2010 792

WEST BOUND

IMPROVED LANE GEOMETRICS

NORTH BOUND LL TTTT R

SOUTH BOUND LL TTT R

EAST BOUND LL TTTT R

LLL TTT R

TRAFFIC SCENARIOS

**GEOMETRICS** 

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

, manufacture of the contract			LE	VEL OF SE	RVICE CALCULATIO	NS				 
MOVE-	# OF	9 - 2		SCENARIO \	VOLUMES			SCENARIO V	//C RATIOS	
MENTS	LANES	CAPACITY	1	2 3	4 .	1	2	3	4	
NBL	2	3200		419	420			0.131 *	0.131 *	
NBT	4	6400		2480	2480			0.388	0.388	
NBR	1	1600		90	90			0.056	0.056	
SBL SBT SBR	2 4 1	3200 6400 1600		190 2010 792	190 2010 798	۲,	<u>s</u>	0.059 0.314 * 0.495	0.059 0.314 * 0.499	
EBL EBT EBR	2 4 1	3200 6400 1600		975 570 337	980 570 338			0.305 0.089 * 0.211	0.306 0.089 * 0.211	
WBL WBT WBR	3 3 1	4800 4800 1600		710 1091 520	710 1091 520			0.148 0.227 * 0.325	0.148 0.227 * 0.325	
			1		LOST TIME:			0.00	0.00	
		TC			ACITY UTILIZATION: EL OF SERVICE:			0.76 C	0.76 C	

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

ROSE AVENUE

E/W STREET:

U.S. HIGHWAY 101 NORTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

A STATE OF THE STA				Т	RAFFIC '	VOLU <i>l</i>	ME SUA	MARY					
	NOF	RTH BOL	JND	SOL	JTH BOL	JND	EAS	T BOUN	1D	WE:	ST BOUN	D	
VOLUMES	L	Т	R	L	T	R	L	Т	R	L	T	R	
(A) EXISTING:	0	1212	0	0	1086	0	0	0	0	338	0	93	
(B) PROJECT-ADDED:	0	4	0	0	1	0	0	0	0	0	0	0	
(C) CUMULATIVE:	0	1363	0	0	1237	0	0	0	0	403	0	143	

GEOMETRICS

LANE GEOMETRICS

NORTH BOUND TTT R

SOUTH BOUND TTT R

EAST BOUND

**WEST BOUND** 

REF: 4 AM

L LR

#### TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SE	RVICE CA	ALCULATION	NS					
1401/5	# OF			SCE	NARIO \	VOLUMES			9	CENARIO V	//C RATIOS		
MOVE-	LANES	CAPACITY	1	2	3	4		1	2	3	4		
MENTS	LAINES	CALACIT	·								_		
NBL	0	0	0	0	0	0		- 0.053 *	0.253 *	0.284 *	0.285 *		
NBT	3	4800	1212	1216	1363	1367		0.253 *		0.000	0.000		
NBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000		
											_		
SBL	0	0	0	0	0	0		-	-	- 0.250	0.258		
SBT	3	4800	1086	1087	1237	1238		0.226	0.226	0.258	0.238		
SBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000		
EBL	0	0	0	0	0	0		-	-	-			
EBT	0	0	0	0	0	0		-	-	-	-		
EBR	0	0	0	0	0	0		-	-	-	-		
		-											
WBL	0	0 .	338	338	403	403		-	-	-	-		
WBT	3	4800	0	0	0	0		0.090 *	0.090 *	0.114 *	0.114 *		
WBR	0	0	93	93	143	143		-	-	-	-		
7,51													
						LOS	T TIME:	0.00	0.00	0.00	0.00		
		TC	TAI INT	DSECTIO	ON CAPA	CITY UTI	LIZATION:	0.34	0.34	0.40	0.40		
		10	JIAL INTI			EL OF SER		A	A	A	A		
1				SCENA	IKIO LEV	LL OF JEN	YICL		.,			and a make the same to	
MOTEC			AND DESCRIPTION OF STREET	and the second second second	to the last state of								

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

ROSE AVENUE

E/W STREET:

U.S.HIGHWAY 101 NORTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

				Т	RAFFIC	VOLU <i>l</i>	ME SU	MMARY					
	NOI	RTH BOL	JND	SOL	JTH BOL	JND	EAS	T BOUN	1D	WE	st boun	D	
VOLUMES	L	T	R	L	Т	R	L	T	R	L	T	R	
(A) EXISTING:	0	1785	0	0	1382	0	0	0 .	0	323	0	817	
(B) PROJECT-ADDED:	0	2	0	0	3	0	0	0	0	0	0	0	
(C) CUMULATIVE:	0	1447	0	0	1432	0	0	0	0	373	0	902	

#### GEOMETRICS

NORTH BOUND LANE GEOMETRICS

SOUTH BOUND TTT R

EAST BOUND

WEST BOUND

REF: 4 PM

L LR

TTT R

#### TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A + B)

SCENARIO 3 = CUMULATIVE(C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVE	L OF SE	RVICE C	ALCULATION	NS .				
MOVE-	# OF			SCE	NARIO V	VOLUME:	5			SCENARIO \	//C RATIOS	
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4	
NBL	0	0	0	0	0	0		_	_	_	_	
NBT	3	4800	1785	1787	1447	1449		0.372 *	0.372 *	0.301 *	0.302 *	
NBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000	
INDK	1	1000	"	U	O	Ü		0.000	0.000			
SBL	0	0	0	0	0	0		-	-	-	-	
SBT	3	4800	1382	1385	1432	1435		0.288	0.289	0.298	0.299	
SBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000	
52.1												
EBL	0	0	0	0	0	0			-	-		
EBT	0	0	0	0	0	0		-	-	-		
EBR	0	0	0	0	0	0		-		-	-	
WBL	0	0	323	323	373	373		-	-	-	-	
WBT	3	4800	0	0	0	0		0.238 *	0.238 *	0.266 *	0.266 *	
WBR	0 .	0	817	817	902	902		-	-	- "	· -	
		1					T 711.15	0.00	0.00	0.00	0.00	
						LOS	T TIME:	0.00	0.00	0.00	0.00	
8 8			~	DCFCT: C	NI CAR:	CITY L'T	LIZATION	0.61	0.61	0.57	0.57	
		10	IALINIE				LIZATION:			A A	A A	
				SCENA	KIO LEVI	EL OF SER	(VICE:	В	В	A	^	
			are the state of t				A STATE OF THE STATE OF	and the second second second				

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

ROSE AVENUE

E/W STREET:

U.S. HIGHWAY 101 SOUTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

The state of the s				T	RAFFIC '	VOLU	ME SUN	MARY	,				 
	NOI	RTH BOL	JND	SOL	JTH BOL	JND	EAS	BOU	ND	WE	ST BOUNI	D	
VOLUMES	1	T	R	L	Т	R	L	Τ	R	L	T	R	
(A) EXISTING:	0	1741	0	0	1258	0	222	0	573	0	0	0	
(B) PROJECT-ADDED:	0	19	0	0	1	0	0	0	4	0	0	0	
(C) CUMULATIVE:	0	1916	0	0	1308	0	272	0	640	0	0	0	

GEOMETRICS

NORTH BOUND

SOUTH BOUND

EAST BOUND

WEST BOUND

REF: 5 AM

LANE GEOMETRICS

TTT R

TTT R

L LR R

#### TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	ALCULATION	NS				 		
MOVE-	# OF			SCE	NARIO \	OLUMES	<u>S</u>		2	SCENARIO V	//C RATIOS	
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4	 
MEINTS	LAINLS			_						_	_	
NBL	0	0	0	0	0	0		0.363 *	0.367 *	0.399 *	0.403 *	
NBT	. 3	4800	1741	1760	1916	1935		0.000	0.000	0.000	0.000	
NBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000	
											_	
SBL	0	0	0	0	0	0		- 0.060	0.262	0.273	0.273	
SBT	. 3	4800	1258	1259	1308	1309		0.262	0.262	0.273	0.000	
SBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000	
EBL	0	0	222	222	272	272		-	- *	0.100 *	0.191 *	
EBT	3	4800	0	0	0	0		0.166 *	0.166 *	0.190 *	1	
EBR	0	0	573	577	640	644		-	-	-	-	
WBL	0	0	0	0	0	0		-	-	-	-	
WBT	0	0	0	0	0	0		-	-	-	-	
WBR	0	0	0	0	0	. 0		-	-	- '	-	
										200 - 200 - 200		
						LOS	ST TIME:	0.00	0.00	0.00	0.00	
		TO	TAL INT	ILIZATION:	0.53	0.53	0.59	0.59				
		10	TAL INT	RVICE:	A	A	A	A				
				JCLINA	INIO LLV	LL OI JL						
NOTES:	to the second second											

NOTES:

Printed:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

**ROSE AVENUE** 

E/W STREET:

U.S.HIGHWAY 101 SOUTHBOUND RAMPS

CONTROL TYPE:

SIGNAL

TRAFFIC VOLUME SUMMARY													
NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND												O	
VOLUMES	L	T	R	L	Т	R	L	T	R	L	T	R	
(A) EXISTING:	0	1785	0	0	1382	0	323	0	817	0	0	0	
(B) PROJECT-ADDED:	0	10	0	0	3	0	0	0	13	0	0	0	
(C) CUMULATIVE:	0	1985	0	0	1432	0	373	0	902	0	0	0	

GEOMETRICS

LANE GEOMETRICS

NORTH BOUND TTT R SOUTH BOUND TTT R EAST BOUND L LR R WEST BOUND

REF: 5 PM

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEI	. OF SE	RVICE C	CALCULATION	NS					
MOVE-	# OF			SCE	NARIO \	OLUME	<u>s</u>			SCENARIO \	//C RATIOS		
MENTS	LANES	CAPACITY	1	2	3	4		. 1	2	3	4		
NBL	0	0	0	. 0	0	0		-	-	-	-		
NBT	3	4800	1785	1795	1985	1995		0.372 *	0.374 *	0.414 *	0.416 *		
NBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000		
NOK		1000		_									
SBL	. 0	0	0	0	0	0		_		-	-		
SBT	3	4800	1382	1385	1432	1435		0.288	0.289	0.298	0.299		
SBR	1	1600	0	0	0	0		0.000	0.000	0.000	0.000		
JUK			-										
EBL	0	0	323	323	373	373		-	-	i-	-		1.
EBT	3	4800	0	0	0	0		0.238 *	0.240 *	0.266 *	0.268 *		
EBR	0	0	817	830	902	915		-	-1	-	-		
WBL	0	0	0	0	0	0		-	-	- '	-		
WBT	0	0	0	0	0	0			-	-	-		
WBR	0	0	0	0	0	0		-	-	-	-		
						LOS	ST TIME:	0.00	0.00	0.00	0.00		
		TO	TAL INTE	ILIZATION:	0.61	0.61	0.68	0.68					
l				SCENA		В	В	В	В				
												100	

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

ROSE AVENUE

E/W STREET:

LOCKWOOD STREET

CONTROL TYPE:

SIGNAL

TRAFFIC VOLUME SUMMARY														
	NOF	RTH BOU	JND	SOU	TH BOL	JND	EAS	BOUN	1D	WE	ST BOUNI	)		
VOLUMES	L	Т	R	L	T	R	L	T	R	L	T	R		
(A) EXISTING: (B) PROJECT-ADDED: (C) CUMULATIVE:	60 0 65	1719 0 1959	28 0 33	319 4 327	1305 0 1535	162 0 167	183 0 188	49 1 56	63 0 68	28 0 33	36 1 42	269 19 280		

# **GEOMETRICS**

NORTH BOUND LL TT TR

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 06 AM

LANE GEOMETRICS

LL TTT R

LLTR

LLTR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

	the state of the s			LEVEL	OF SEI	RVICE CA	LCULATION	IS				 
MOVE-	# OF			SCE	NARIO \	OLUMES			2	SCENARIO V	//C RATIOS	- 1
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4	 
MENTS	LAIYLS							0.019	0.019	0.020	0.020	
NBL	2	3200	60	60	65	65			0.364 *	0.415 *	0.415 *	1
NBT	3	4800	1719	1719	1959	1959		0.364 *	0.364	0.415	0.415	
NBR	0	0	28	28	33	33		-	-	-	-	
										0.400 *	0.103 *	
SBL	2	3200	319	323	327	331		0.100 *	0.101 *	0.102 *	200300000000000000000000000000000000000	
SBT	3	4800	1305	1305	1535	1535		0.272	0.272	0.320	0.320	
SBR	1	1600	162	162	167	167		0.101	0.101	0.104	0.104	
SBIK												
EBL	2	3200	183	183	188	188		0.057 *	0.057 *	0.059 *	0.059 *	
	1	1600	49	50	56	57		0.031	0.031	0.035	0.036	
EBT		1600	63	63	68	68		0.039	0.039	0.043	0.043	
EBR	1	1000										-
		0	28	28	33	33		-	-	-1	-	
WBL	0		36	37	42	43		0.020	0.020	0.023	0.024	
WBT	2	3200		58	56	60		0.034 *	0.036 *	0.035 *	0.038 *	
WBR	1	1600	54	50	30			0.031				
						105	F TU 15.	0.00	0.00	0.00	0.00	
						LOS	TTIME:	0.00	0.00	0.00		
		то	TAL INTE	RSECTIO	ON CAPA	CITY UTI	LIZATION:	0.56	0.56	0.61	0.62	
						EL OF SER		A	A	В	В	
							And the second s					

NOTES:Westbound Right-Turn Overlap with Southbound Left-Turn

10/12/22

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

ROSE AVENUE LOCKWOOD STREET

E/W STREET: CONTROL TYPE:

SIGNAL

								No. of Concession, Name of Street, or other Designation, or other	The second second	AND RESIDENCE OF THE PARTY OF T	the same of the same of the	Charles and the second state of the second sta	
				TI	RAFFIC	VOLU	ME SUN	<i>M</i> ARY					
	NOR	TH BOL	JND	SOL	JTH BO	JND	EAS	T BOUN	1D	WES	T BOUN	D	
VOLUMES	L	T	R	L	T	R	L	T	R	L	T	R	
(A) EXISTING:	231	1348	44	305	1486	391	392	93	199	117	80	359	
(B) PROJECT-ADDED:	0	0	0	16	0	0	0	1	0	0	1	10	
(C) CUMULATIVE:	241	1628	54	321	1731	401	402	104	209	127	91	374	

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 06 PM

LANE GEOMETRICS

LL TT TR

LL TTT R

LLTR

LLTR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	. OF SE	RVICE C.	ALCULATION	١S					
	# OF			SCE	NARIO \	OLUMES	5			SCENARIO V	//C RATIOS		
MOVE-	# OF	CAPACITY	1	2	3	. 4	-	1	2	3	4		
MENTS	LAINES	CALACIT						0.070	0.072	0.075	0.075		
NBL	2	3200	231	231	241	241		0.072	0.072		0.350 *		
NBT	3	4800	1348	1348	1628	1628		0.290 *	0.290 *	0.350 *	0.330		1
NBR	0	0	44	44	54	54		-	-	-	-		
											0.405 *		
SBL	2	3200	305	321	321	337		0.095 *	0.100 *	0.100 *	0.105 *		
SBT	3	4800	1486	1486	1731	1731		0.310	0.310	0.361	0.361		
SBR	1	1600	391	391	401	401		0.244	0.244	0.251	0.251		
5511.													
EBL	2	3200	392	392	402	402		0.123 *	0.123 *	0.126 *	0.126 *		
EBT	1	1600	93	94	104	105		0.058	0.059	0.065	0.066		
EBR	1	1600	199	199	209	209		0.124	0.124	0.131	0.131		
EDK	. '												
WDI	0	0	117	117	127	127		-	-	-	-		
WBL	2	3200	80	81	91	92		0.062	0.062	0.068	0.068		
WBT	1	1600	197	203	206	211		0.123 *	0.127 *	0.129 *	0.132 *		
WBR		1000	137										
						105	T TIME:	0.00	0.00	0.00	0.00	-	
						203	, ,,,,,,						
							0.71	0.71					
		TO	TAL INTE	LIZATION:	0.63	0.64	0.71	0.71					
1				RVICE:	В	В	С	С					
				1	7		The second second			1000			

NOTES: Westbound Right-Turn overlap with Southbound Left-Turn

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

**ROSE AVENUE** GONZALEZ ROAD

E/W STREET: CONTROL TYPE:

SIGNAL

CONTROLINE					_	and the same of the same							
				TF	RAFFIC	VOLUN		MARY			T DOLLNE		 
	NOR	TH BOU	IND	SOU	TH BO	JND	EAS	T BOUN	ID	WE:	ST BOUNE	)	
VOLUMES	I	T	R	L	Т	R	L	T	R	L	T	R	
VOLUMES  (A) EXISTING: (B) PROJECT-ADDED: (C) CUMULATIVE:	322 0 325	1114 0 1234	180 1 281	274 0 334	926 0 1026	263 0 323	644 0 724	807 5 1974	262 0 292	72 4 123	437 15 678	115 0 165	

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 07 AM

LANE GEOMETRICS

LL TTT R

LL TTT R

LLL TT R

L TTT R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEI	. OF SEI	RVICE CALCULAT	IONS						
	".05					VOLUMES			<u>S</u>	CENARIO V	//C RATIOS		
MOVE-	# OF	CAPACITY	1	2	3	4		1	2	3	4		
MENTS	LANES	CAPACITI						101	0.101				
NBL	2	3200	322	322	325	325		0.101	0.101				
NBT	3	4800	1114	1114	1234	1234		0.232 *	0.232				
NBR	1	1600	180	181	281	282	0	0.113	0.113				
								0.086 *	0.086 *				
SBL	2 .	3200	274	274	334	334			0.193				
SBT	3	4800	926	926	1026	1026		0.193	0.193				
SBR ·	1	1600	263	263	323	323	10	0.164	0.164				
								0.40	0.13				
EBL	3 .	4800	644	644	724	724	- 1	0.13					
EBT	2	3200	807	812	1974	1979	1	0.252 *	0.254 *			/	
EBR	1	1600	262	262	292	292		0.16	0.16				
							١,	0.045 *	0.048 *				
WBL	1	1600	72	76	123	127		0.045 *	0.048				
WBT	3	4800	437	452	678	693	- 1	0.091	0.0000000000000000000000000000000000000				
WBR	1	1600	115	115	165	165	,	0.072	0.072				
								0.00	0.00				
						LOST TIME:	12	0.00	0.00				
		TC	TAL INTE	RSECTIO	ON CAPA	ACITY UTILIZATION	:	0.62	0.62				
						'EL OF SERVICE:		В	В				
					100	and the state of t		Trefficience.	A STATE OF THE STA	The Park Street	The second	man, does not have been	

NOTES:

10/12/22

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

**ROSE AVENUE** GONZALEZ ROAD

E/W STREET:

SIGNAL

CONTROL	TYPE:	SIGNAL
	To be the second	and which has been to

TRAFFIC VOLUME SUMMARY													
	NOR	RTH BOL	JND	SOU	TH BOI	JND	EAS	BOUN	1D	WE	st bound	)	
VOLUMES	L	Т	R	L	T	R	L	T	R	L	T	R	
(A) EXISTING: (B) PROJECT-ADDED: (C) CUMULATIVE:	430 0 430	1047 0 1197	95 3 196	147 0 197	1246 0 1366	460 0 560	462 0 562	454 12 950	153 0 196	183 2 234	943 8 2001	169 0 219	

GEOMETRICS

NORTH BOUND

SOUTH BOUND

EAST BOUND

WEST BOUND

REF: 07 PM

LANE GEOMETRICS

LL TTT R

LL TTT R

LLL TT R

L TTT R

#### TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

Commission of the Commission o				LEVE	OF SE	RVICE CALCULATIO	NS				
MOVE-	# OF			SCE	NARIO V	VOLUMES		9	Scenario V	//C RATIOS	
MENTS	LANES	CAPACITY	1	2	3	4	1	2	3	4	 
MEINIS			420	430	430	430	0.134 *	0.134 *			
NBL	2	3200	430			1197	0.218	0.218			
NBT	3	4800	1047	1047	1197		0.059	0.061	V .		
NBR	1	1600	95	98	196	199	0.055	0.001			1
				4.77	107	197	0.046	0.046			
SBL	2	3200	147	147	197		0.260 *	0.260 *		y 2	
SBT	3	4800	1246	1246	1366	1366	0.288	0.288			
SBR	1	1600	460	460	560	560	0.200	0.200			
	1						0.096 *	0.096 *			
EBL	3	4800	462	462	562	562		0.096	l		
EBT	2	3200	454	466	950	962	0.142	LESSON NUC			
EBR	1	1600	153	153	196	196	0.096	0.096			
								0.446			
WBL	1	1600	183	185	234	236	0.114	0.116			
WBT	3	4800	943	951	2001	2009	0.196 *	0.198 *			
WBR	1	1600	169	169	219	219	0.106	0.106			
						LOST TIME:	0.00	0.00			
		TC	TAL INTE	FRSECTIO	ON CAPA	CITY UTILIZATION:	0.69	0.69			
1		10	ALAE II (II			EL OF SERVICE:	В	В			
				332117							
MOTEC	the same of the sa										

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

ROSE AVENUE

E/W STREET:

**GONZALEZ ROAD** 

CONTROL TYPE:

SIGNAL

100					TF	RAFFIC	VOLU	ME SU	MMARY					
NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND  VOLUMES L T R L T R L T R														
VOI	LUMES	L	Т	R	L	T	R	L	T	R	L	T	R	
(A)	EXISTING:	322	1114	180	274	926	263	644	807	262	72	437	115	
(B)	PROJECT-ADDED:	0	0	1	0	0	0	0	5	0	4	1	0	
(C)	CUMULATIVE:	325	1234	281	334	1026	323	724	1974	292	123	678	165	

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 07 AM

IMPROVED LANE GEOMETRICS

LL TTT R

LL TTTT R

LLL TTT R

LL TTTT R

## TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVE	OF SE	RVICE CALCU	LATION	NS				 
MOVE-	# OF			SCE	NARIO \	/OLUMES				SCENARIO V	//C RATIOS	
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4	
	0	3200	322	322	325	325		0.101	0.101	0.102	0.102	
NBL	2		1114	1114	1234	1234		0.232 *	0.232 *	0.257 *	0.257 *	
NBT	3	4800			281	282		0.113	0.113	0.176	0.176	
NBR	1	1600	180	181	201	202		0.113	0.115	0.170	0,,,,	
SBL	2	3200	274	274	334	334		0.086 *	0.086 *	0.104 *	0.104 *	
SBT	4	6400	926	926	1026	1026		0.145	0.145	0.160	0.160	
SBR	1	1600	263	263	323	323		0.164	0.164	0.202	0.202	
SDK		1,000										
EBL	3	4800	644	644	724	724		0.134 *	0.134 *	0.151	0.151	
EBT	3	4800	807	812	1974	1979		0.168	0.169	0.411 *	0.412 *	
EBR	1	1600	262	262	292	292		0.164	0.164	0.183	0.183	
LDK												
WBL	2	3200	72	76	123	127		0.023	0.024	0.038 *	0.040 *	
WBT	4	6400	437	438	678	679		0.068	0.068	0.106	0.106	
WBR	1	1600	115	115	165	165		0.072 *	0.072 *	0.103	0.103	
						LOST TIME:	:	0.00	0.00	0.00	0.00	
		TO	TAL INTE	DSECTIO	N CAPA	CITY UTILIZAT	ION:	0.52	0.52	0.81	0.81	
		10	TAL HATE			EL OF SERVICE:		A	A	D	D	
				JCLINA	NO LLV	LE OF SERVICE.		7.				

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

ROSE AVENUE **GONZALEZ ROAD** 

E/W STREET:

CONTROL TYPE:	SIGNAL							PA MIN ON						a 2 C-11	
						RAFFIC					\A/E(	T BOUNE	`		
		NOR	TH BOU	JND	SOL	ITH BO	JND	EAS	r Boun	1D	VVES	I ROOME	,		
VOLUMES		L	T	R	L	T	R	L	T	R	L	T	R		
(A) EXISTING: (B) PROJECT-ADD (C) CUMULATIVE:		430 0 430	1047 0 1197	95 3 196	147 0 197	1246 0 1366	460 0 560	462 0 562	454 12 950	153 0 196	183 2 234	943 8 2001	169 0 219		

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

IMPROVED LANE GEOMETRICS

LL TTT R

LL TTTT R

LLL TTT R

LL TTTT R

REF: 07 PM

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A + B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVE	OF SE	RVICE CAL	CULATION	NS					
MOVE-	# OF			SCE	NARIO V	VOLUMES			2	CENARIO V	//C RATIOS		
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4		
MEINIS			120	420	420	430		0.134 *	0.134 *	0.134 *	0.134 *	-	
NBL	2	3200	430	430	430			0.134	0.134	0.249	0.249		
NBT	3	4800	1047	1047	1197	1197	w.		0.210	0.123	0.124		
NBR	1	1600	95	98	196	199		0.059	0.061	0.123	0.124		
			-					0.046	0.046	0.062	0.062		
SBL	2	3200	147	147	197	197				0.002	0.213 *		
SBT	4	6400	1246	1246	1366	1366		0.195 *	0.195 *				
SBR	1	1600	460	460	560	560		0.288	0.288	0.350	0.350		
EBL	3	4800	462	462	562	562		0.096 *	0.096 *	0.117 *	0.117 *		
EBT	3	4800	454	466	950	962		0.095	0.097	0.198	0.200	,	
EBR	1	1600	153	153	196	196		0.096	0.096	0.123	0.123		
LDK													
WBL	2	3200	183	185	234	236		0.057	0.058	0.073	0.074		
WBT	4	6400	943	951	2001	2009		0.147 *	0.149 *	0.313 *	0.314 *		
WBR	1	1600	169	169	219	219		0.106	0.106	0.137	0.137		
VVDK		1000							-				
						LOST TI	IME:	0.00	0.00	0.00	0.00		
						CITY LITTE 17	ATION.	0.57	0.57	0.78	0.78		
1		TC	TAL INTE			CITY UTILIZ		200		C	C		
				SCENA	RIO LEV	EL OF SERVI	CE:	A	A				
Albert John Commission	er, spine for regent		at a supplied to the	and the second				And the same of the same of					

NOTES:

Printed:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

WILIAMS DRIVE

E/W STREET:

GONZALEZ ROAD SIGNAL

CONTROL TYPE:	SIGNAL							Mark and the second						
		NOP	TH BO	IND		RAFFIC JTH BO			MMARY T BOUN		WE	ST BOUNE	)	 
VOLUMES		L	T	R	L	T	R	L	Т	R	L	T	R	
(A) EXISTING: (B) PROJECT-ADDED (C) CUMULATIVE:	):	186 0 186	176 0 176	54 0 54	88 0 88	71 0 71	17 0 17	83 0 83	989 6 2317	127 0 127	70 0 70	431 19 773	118 0 118	

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

WEST BOUND

REF: 08 AM

LANE GEOMETRICS

L TR

LTR

LTTTR

L TT TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A + B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SEF	RVICE C	ALCULATION	NS .					
MOVE-	# OF			SCE	NARIO V	OLUMES	5		2	Scenario V	//C RATIOS		
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4		
NBL	1	1600	186	186	186	186		0.116	0.116	0.116	0.116	-	9
NBT	1	1600	176	176	176	176		0.144 *	0.144 *	0.144 *	0.144 *		
NBR	0	0	54	54	54	54		-	-	-	-		
SBL SBT SBR	1 1 1	1600 1600 1600	88 71 17	88 71 17	88 71 17	88 71 17		0.055 * 0.044 0.01	0.055 * 0.044 0.01	0.055 * 0.044 0.011	0.055 * 0.044 0.011		
EBL EBT EBR	1 3 0	1600 4800 0	83 989 127	83 995 127	83 2317 127	83 2323 127		0.052 0.233 * -	0.052	0.052 0.509 * -	0.052 0.510 *		
WBL WBT WBR	1 3 0	1600 4800 0	70 431 118	70 450 118	70 773 118	70 792 118		0.044 * 0.114 -	0.044 * 0.118 -	0.044 * 0.186	0.044 * 0.190		
*.		TC	DTAL INTE				ST TIME: ILIZATION: RVICE:	0.00 0.48 A	0.00 0.48 A	0.00 0.75 C	0.00 0.75 C		

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR WILIAMS DRIVE

N/S STREET:

GONZALEZ ROAD

E/W STREET: CONTROL TYPE:

SIGNAL

CONTROLITIES								A 100	and the same of the same of		A LANGE TO SERVICE OF THE PARTY	and the second second second second	man of the last of	
and the second of the second of the second	AND THE PARTY OF T				TF	RAFFIC	VOLUA	ME SU	MMARY					 
		NOR	TH BO	UND	SOU	TH BO	UND	EAS	T BOUN	ID	WE	ST BOUND	)	
volumes		1	T	R	L	Т	R	L	T	R	L	T	R	
VOLUMES										or	87	1000	156	
(A) EXISTING:		152	146	59	149	159	116	70	571	95		1000	0	
(B) PROJECT-ADDE	D:	0	0	0	0	0	0	0	15	0	0	2159	156	
(C) CUMULATIVE:		152	146	59	149	159	116	70	1218	95	87	2159	130	

**GEOMETRICS** 

NORTH BOUND SOUTH BOUND EAST BOUND

WEST BOUND

REF: 08 PM

LANE GEOMETRICS

L TR

LTR

L TT TR

L TT TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SER	RVICE CALCULAT	IONS				 
1401/5	# OF			SCE	NARIO V	OLUMES			SCENARIO V	//C RATIOS	
move- ments	LANES	CAPACITY	1	2	3	4	1	2	3	4	
		1600	152	152	152	152	0.095	0.095	0.095	0.095	
NBL	1	1600	146	146	146	146	0.128	* 0.128 *	0.128 *	0.128 *	
NBT NBR	0	0	59	59	59	59	-	-	-	- 1	
SBL SBT SBR	1 1 1	1600 1600 1600	149 159 116	149 159 116	149 159 116	149 159 116	0.093 0.099 0.073	* 0.093 * 0.099 0.073	0.093 * 0.099 0.073	0.093 * 0.099 0.073	
EBL EBT EBR	1 3 0	1600 4800 0	70 571 95	70 586 95	70 1218 95	70 1233 95	0.044 0.139 -	* 0.044 * 0.142 -	0.044 * 0.274	0.044 * 0.277 -	
WBL WBT WBR	1 3 0	1600 4800 0	87 1000 156	87 1010 156	87 2159 156	87 2169 156	0.054 0.241 -	* 0.054 * 0.243 * -	0.054 0.482 * -	0.054 0.484 * -	
			1			LOST TIME:	0.00	0.00	0.00	0.00	
	,	тс	OTAL INTE			CITY UTILIZATION EL OF SERVICE:	: 0.51 A	0.51 A	0.75 C	0.75 C	

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET: E/W STREET: LOMBARD STREET GONZALEZ ROAD

CONTROL TYPE:

SIGNAL

CONTROLITIES						C. A. Street and Street Co.		on the property of	The second section in		State of the latest state of		
	The last of the Spinger	and his year of the	1	TI	RAFFIC	VOLU	ME SUN	MARY					 
	NOR	TH BO	UND		TH BO			T BOUN		WES	ST BOUND	)	
VOLUMES	L	T	R	L	Т	R	L	T	R	L	T	R	
(A) EXISTING: (B) PROJECT-ADDED: (C) CUMULATIVE:	146 0 146	57 0 57	85 0 85	29 21 39	15 0 15	39 19 47	178 6 187	733 0 2058	208 0 208	156 0 156	447 0 807	55 6 62	

SOUTH BOUND EAST BOUND
LT TR LL TTT R L TTT R

REF: 9 AM

TRAFFIC SCENARIOS

GEOMETRICS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

LANE GEOMETRICS

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

NORTH BOUND

L TT R

				LEVEL	OF SEI	RVICE CA	LCULATION	NS				
	# OF			SCE	NARIO V	OLUMES			2	CENARIO V	//C RATIOS	
MOVE-		CAPACITY	1	2	3	4		1	2	3	4	
MENTS	LANES 1	1600	146	146	146	146		0.091 *	0.091 *	0.091 *	0.091 *	
NBL		3200	57	57	57	57		0.018	0.018	0.018	0.018	
NBT	2		85	85	85	85		0.053	0.053	0.053	0.053	
NBR	1	1600	0.5	03	03	03						
SBL	0	0	29	50	39	60		-	-	-	-	
	2	3200	15	15	15	15		0.026 *	0.038 *	0.032 *	0.044 *	
SBT	0	0	39	58	47	- 66		-	-	-	-	
SBR	0											
		3200	178	184	187	193		0.056	0.058	0.058	0.060	
EBL	2		733	733	2058	2058		0.153 *	0.153 *	0.429 *	0.429 *	
EBT	3	4800		208	2030	208		0.130	0.130	0.130	0.130	
EBR	1	1600	208	208	200	200		05				
WBL	1	1600	156	156	156	156		0.098 *	0.098 *	0.098 *	0.098 *	
WBT	3	4800	447	447	807	807		0.093	0.093	0.168	0.168	
WBR	1	1600	55	61	62	68		0.034	0.038	0.039	0.043	
VVDIC	<u></u>					LOST	T TIME:	0.00	0.00	0.00	0.00	
		TC	OTAL INTE	RSECTIC SCENA	LIZATION: VICE:	0.37 A	0.38 A	0.65 B	0.66 B			
MOTES	and the state of the state of		to Alaska to	water to be the	World Co.							

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

LOMBARD STREET GONZALEZ ROAD

E/W STREET:

SIGNAL

CONTROL TYPE:	SIGNAL									As The Control			
		NOR	TURO	LINID		RAFFIC JTH BO	VOLUM		MMARY T BOUN		WES	ST BOUND	)
VOLUMES		NOR L	TH BO	R	L	Т	R	L	T	R	L	Т	R
(A) EXISTING: (B) PROJECT-ADDEC	D:	181 0 181	62 0 62	78 0 78	73 10 82	29 0 29	192 10 202	146 15 157	537 0 1178	117 0 117	115 0 115	895 0 2049	35 16 44

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND

EAST BOUND

**WEST BOUND** 

REF: 9 PM

LANE GEOMETRICS

LTTR

LT TR

LL TTT R

L TTT R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SE	RVICE CALCULATION	VS ·				 
LIOVE	# OF			SCE	NARIO \	/OLUMES		2	CENARIO V	//C RATIOS	
move- ments	LANES	CAPACITY	1	2	3	4	1	2	3	4	 
MIEINIS	LAINES		101	101	181	181	0.113 *	0.113 *	0.113 *	0.113 *	
NBL	1	1600	181	181	62	62	0.019	0.019	0.019	0.019	
NBT	2	3200	62	62			0.049	0.049	0.049	0.049	
NBR	1	1600	78	78	78	78	0.043	0.015	0.0		
								_	_	_	
SBL	0	0	73	83	82	92	0.092 *	0.098 *	0.098 *	0.104 *	
SBT	2	3200	29	29	29	29	0.092	0.030	-	_	
SBR	0	0	192	202	202	212	-	-			
							0.046 *	0.05 *	0.049 *	0.054 *	1
EBL	2	3200	146	161	157	172	0.046 *		0.049	0.245	
EBT	3	4800	537	537	1178	1178	0.112	0.112		0.073	
EBR	1	1600	117	117	117	117	0.073	0.073	0.073	0.073	
									0.070	0.072	
WBL	1	1600	115	115	115	115	0.072	0.072	0.072	0.072	
WBT	3	4800	895	895	2049	2049	0.186 *	0.186 *	0.427 *	1	
WBR	1	1600	35	51	44	60	0.022	0.032	0.028	0.038	
										0.00	
						LOST TIME:	0.00	0.00	0.00	0.00	
		TC	TAI INTE	RSECTIO	N CAPA	CITY UTILIZATION:	0.44	0.45	0.69	0.70	
		IC	ALVE HALE			EL OF SERVICE:	A	Α	В	В	
l				SCEINA	NIO LLV	LE OF SERVICES		The state of the s			
NOTES:	the state of the state of		The same of the sa								

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

AM PEAK HOUR

N/S STREET:

SOLAR DRIVE

E/W STREET:

GONZALEZ ROAD

CONTROL TYPE:

SIGNAL

								MARY		\\/F	ST BOUNE	)	
VOLUMES	NOR I	TH BO	UND R	SOU	TH BO	UND R	L L	T BOUN T	R	L	T	R	
(A) EXISTING: (B) PROJECT-ADDED:	28	37	108	70 2	13	21	126 0	540 21	55 0	300	573 6	171 2	
(C) CUMULATIVE:	28	37	108	72	13	21	126	1870	55	300	911	173	

## **GEOMETRICS**

NORTH BOUND

SOUTH BOUND

**EAST BOUND** 

**WEST BOUND** 

REF: 10 AM

LANE GEOMETRICS

LTR

L TR

L TT TR

L TT TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A + B)

SCENARIO 3 = CUMULATIVE(C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SEF	RVICE C.	ALCULATION	٧S					
MOVE-	# OF			SCE	NARIO V	OLUMES	5	SCENARIO V/C RATIOS					
MENTS	LANES	CAPACITY	1	2	3	4		1	2	3	4		
MEINTS	LAIRLS							0.018	0.018	0.018	0.018		
NBL	1	1600	28	28	28	28		0.018	0.013	0.013	0.023		2
NBT	1	1600	37	37	37	37			0.023	0.023	0.068 *		
NBR	1	1600	108	108	108	108		0.068 *	0.068	0.066	0.000		
		1600	70	72	72	74		0.044 *	0.045 *	0.045 *	0.046 *		
SBL		1600	13	13	13	13		0.021	0.021	0.021	0.021	-0	
SBT	1		21	21	21	21		-	-	-	-		
SBR	0	. 0 .	21	21	21	~.							
	1	1600	126	126	126	126		0.079	0.079	0.079	0.079		
EBL	. 1	4800	540	561	1870	1891		0.124 *	0.128 *	0.401 *	0.405 *		
EBT	3	0	55	55	55	55			-	-	- 1		
EBR	0	0		55	00								
WBL	1	1600	300	300	300	300		0.188 *	0.188 *	0.188 *	0.188 *		
WBT	3	4800	573	579	911	917		0.155	0.157	0.226	0.228		
A. C. C. C.	0	0	171	173	173	175		-	-	-	1-		
WBR	0	Ů						-					
						LOS	T TIME:	0.00	0.00	0.00	0.00		
		TC	TAL INTE	DCECTIC	NI CAPA	CITY LIT	ILIZATION:	0.42	0.43	0.70	0.71		
		10	TAL INTE			EL OF SEI		A	A	В	С		
1				SCENA	NIO LEVI	LL OI JLI	(TICL)						

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE:

SEPTEMBER 14, 2022

TIME PERIOD:

PM PEAK HOUR

N/S STREET:

SOLAR DRIVE **GONZALEZ ROAD** 

E/W STREET: CONTROL TYPE:

SIGNAL

			Contract Con									
			TF	RAFFIC	VOLU	ME SUN	MARY					
NI	ORTH BC	NIND	SOU	TH BO	UND	EAS	BOUN	D	WE	ST BOUND		
1.	Т	R	L	Т	R	L	T	R	L	T	R	
									400	741	94	
64	17	246	133	36	82	110	727		193		24	
): 0	0	0	2	0	0	0	10	0	0		3	
	17	246	135	36	82	110	1372	59	193	1899	97	
	64 D: 0	L T  64 17 D: 0 0	D: 0 0 0	NORTH BOUND SOU L T R L 64 17 246 133 D: 0 0 0 2	NORTH BOUND SOUTH BO L T R L T 64 17 246 133 36 D: 0 0 0 2 0	NORTH BOUND SOUTH BOUND L T R L T R  64 17 246 133 36 82 D: 0 0 0 2 0 0	NORTH BOUND SOUTH BOUND EAST L T R L  64 17 246 133 36 82 110  D: 0 0 0 2 0 0 0  110	NORTH BOUND SOUTH BOUND EAST BOUND L T R L T R L T  64 17 246 133 36 82 110 727 D: 0 0 0 2 0 0 0 10	L T R L T R L T R  64 17 246 133 36 82 110 727 59  D: 0 0 0 2 0 0 0 10 0	NORTH BOUND SOUTH BOUND EAST BOUND WE L T R L T R L T R L  64 17 246 133 36 82 110 727 59 193 D: 0 0 0 2 0 0 0 10 0 0  127 259 193	NORTH BOUND SOUTH BOUND EAST BOUND  L T R L T R L T R L T  64 17 246 133 36 82 110 727 59 193 741  D: 0 0 0 2 0 0 0 10 0 0 16  D: 10 0 0 16	NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND  L T R L T R L T R  64 17 246 133 36 82 110 727 59 193 741 94  D: 0 0 0 2 0 0 0 10 0 0 16 3

**GEOMETRICS** 

NORTH BOUND

SOUTH BOUND L TR

EAST BOUND

**WEST BOUND** 

LANE GEOMETRICS

LTR

L TT TR

L TT TR

REF: 10 PM

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

SCENARIO 3 = CUMULATIVE (C)

SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

				LEVEL	OF SEI	RVICE CALCULATION	ONS					
MOVE-	# OF			SCE	NARIO \	/OLUMES		SCENARIO V/C RATIOS				
MENTS	LANES	CAPACITY	1	2	3	4	1	2	3	4		
IMEINTS	LANALS					64	0.040	0.040	0.040	0.040		
NBL	1	. 1600	64	64	64		0.040	0.011	0.011	0.011		
NBT	1 1	1600	17	17	17	17		0.011	0.154 *	0.154 *		
NBR	1	1600	246	246	246	246	0.154 *	0.134	0.134	0.134		
				425	125	137	0.083 *	0.084 *	0.084 *	0.086 *		
SBL	1	1600	133	135	135		0.003	0.074	0.074	0.074		
SBT	1	1600	36	36	36	36		0.074	-	_		
SBR	0	0	82	82	82	82	-	-				
			110	110	110	110	0.069 *	0.069 *	0.069 *	0.069 *		
EBL	1	1600	110		1372	1382	0.164	0.166	0.298	0.300		
EBT	3	4800	727	737		59	0.101	-	_	-		
EBR	0	0	59	59	59	59						
		1600	193	193	193	193	0.121	0.121	0.121	0.121		
WBL	1	1600	741	757	1899	1915	0.174 *	0.178 *	0.416 *	0.420 *		
WBT	3	4800		97	97	100	_	-	-	-		
WBR	0	0	94	97	97	100						
						LOST TIME:	0.00	0.00	0.00	0.00		
1		TC	TAL INTE	RSECTIO	N CAPA	CITY UTILIZATION:	0.48	0.49	0.72	0.73		
		Te				EL OF SERVICE:	A	Α	С	С		
	A TOTAL BUT OFFICE	The second secon			Children						The term of the same	

NOTES:

CALIFORNIA GOVERNMENT CODE SECTION 65915 (p)(1)



## State of California

## **GOVERNMENT CODE**

Section 65915

- 65915. (a) (1) When an applicant seeks a density bonus for a housing development within, or for the donation of land for housing within, the jurisdiction of a city, county, or city and county, that local government shall comply with this section. A city, county, or city and county shall adopt an ordinance that specifies how compliance with this section will be implemented. Except as otherwise provided in subdivision (s), failure to adopt an ordinance shall not relieve a city, county, or city and county from complying with this section.
- (2) A local government shall not condition the submission, review, or approval of an application pursuant to this chapter on the preparation of an additional report or study that is not otherwise required by state law, including this section. This subdivision does not prohibit a local government from requiring an applicant to provide reasonable documentation to establish eligibility for a requested density bonus, incentives or concessions, as described in subdivision (d), waivers or reductions of development standards, as described in subdivision (e), and parking ratios, as described in subdivision (p).
- (3) In order to provide for the expeditious processing of a density bonus application, the local government shall do all of the following:
  - (A) Adopt procedures and timelines for processing a density bonus application.
- (B) Provide a list of all documents and information required to be submitted with the density bonus application in order for the density bonus application to be deemed complete. This list shall be consistent with this chapter.
- (C) Notify the applicant for a density bonus whether the application is complete in a manner consistent with the timelines specified in Section 65943.
- (D) (i) If the local government notifies the applicant that the application is deemed complete pursuant to subparagraph (C), provide the applicant with a determination as to the following matters:
- (I) The amount of density bonus, calculated pursuant to subdivision (f), for which the applicant is eligible.
- (II) If the applicant requests a parking ratio pursuant to subdivision (p), the parking ratio for which the applicant is eligible.
- (III) If the applicant requests incentives or concessions pursuant to subdivision (d) or waivers or reductions of development standards pursuant to subdivision (e), whether the applicant has provided adequate information for the local government to make a determination as to those incentives, concessions, or waivers or reductions of development standards.

- (6) "Total units" or "total dwelling units" means a calculation of the number of units that:
- (A) Excludes a unit added by a density bonus awarded pursuant to this section or any local law granting a greater density bonus.

(B) Includes a unit designated to satisfy an inclusionary zoning requirement of a

city, county, or city and county.

- (p) (1) Except as provided in paragraphs (2), (3), and (4), upon the request of the developer, a city, county, or city and county shall not require a vehicular parking ratio, inclusive of parking for persons with a disability and guests, of a development meeting the criteria of subdivisions (b) and (c), that exceeds the following ratios:
  - (A) Zero to one bedroom: one onsite parking space.
  - (B) Two to three bedrooms: one and one-half onsite parking spaces.

(C) Four and more bedrooms: two and one-half parking spaces.

(2) (A) Notwithstanding paragraph (1), if a development includes at least 20 percent low-income units for housing developments meeting the criteria of subparagraph (A) of paragraph (1) of subdivision (b) or at least 11 percent very low income units for housing developments meeting the criteria of subparagraph (B) of paragraph (1) of subdivision (b), is located within one-half mile of a major transit stop, and there is unobstructed access to the major transit stop from the development, then, upon the request of the developer, a city, county, or city and county shall not impose a vehicular parking ratio, inclusive of parking for persons with a disability and guests, that exceeds 0.5 spaces per unit. Notwithstanding paragraph (1), if a development includes at least 40 percent moderate-income units for housing developments meeting the criteria of subparagraph (D) of paragraph (1) of subdivision (b), is located within one-half mile of a major transit stop, as defined in subdivision (b) of Section 21155 of the Public Resources Code, and the residents of the development have unobstructed access to the major transit stop from the development then, upon the request of the developer, a city, county, or city and county shall not impose a vehicular parking ratio, inclusive of parking for persons with a disability and guests, that exceeds 0.5 spaces per bedroom.

(B) For purposes of this subdivision, "unobstructed access to the major transit stop" means a resident is able to access the major transit stop without encountering natural or constructed impediments. For purposes of this subparagraph, "natural or constructed impediments" includes, but is not limited to, freeways, rivers, mountains, and bodies of water, but does not include residential structures, shopping centers, parking lots, or rails used for transit.

- (3) Notwithstanding paragraph (1), if a development consists solely of rental units, exclusive of a manager's unit or units, with an affordable housing cost to lower income families, as provided in Section 50052.5 of the Health and Safety Code, then, upon the request of the developer, a city, county, or city and county shall not impose vehicular parking standards if the development meets either of the following criteria:
- (A) The development is located within one-half mile of a major transit stop and there is unobstructed access to the major transit stop from the development.

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